

Stormwater Analysis & Design Report - Moo Cow Solar

2446 Victory Highway Coventry, RI 02816

February 1, 2024 Revised April 24, 2024



Prepared For:

EDPR NA Distributed Generation, LLC 100 Park Ave, Suite 2400 New York, New York 10017

Owner/Applicant:

EDPR NA Distributed Generation, LLC 100 Park Ave, Suite 2400 New York, New York 10017

Prepared By:

TRC Engineers, Inc. 10 Hemingway Drive, 2nd Floor East Providence, RI 02915





TABLE OF CONTENTS

1.0	PUR	POSE	1
2.0	EXIS	TING CONDITIONS	1
	2.1	Existing Site Use	1
	2.2	Hydrology	1
	2.3	Wetland Delineation	1
	2.4	Test Pits	2
3.0	PRO	POSED DEVELOPMENT	2
4.0	HYD	ROLOGIC AND HYDRAULIC ANALYSIS	4
	4.1	Methodology	4
	4.2	Points Of Analysis	4
	4.3	Pre-Development Drainage Areas	5
	4.4	Post-Development Drainage Areas	5
	4.5	Proposed Stormwater Design	6
	4.6	Results	8
5.0	STO	RMWATER MANAGEMENT STANDARDS	10
	5.1	Minimum Standard 1: LID Site Planning and Design Strategies	10
	5.2	Minimum Standard 2: Groundwater Recharge	10
	5.3	Minimum Standard 3: Water Quality	11
	5.4	Minimum Standard 4: Conveyance and Natural Channel Protection	11
	5.5	Minimum Standard 5: Overbank Flood Protection	12
	5.6	Minimum Standard 6: Redevelopment and Infill Projects	12
	5.7	Minimum Standard 7: Pollution Prevention	12
	5.8	Minimum Standard 8: Land Uses with Higher Potential Pollutant Loads	12
	5.9	Minimum Standard 9: Illicit Discharges	12
	5.10	Minimum Standard 10: Construction Activity SESC and Pollution Prevention Control Measure	13
	5.11	Minimum Standard 11: Stormwater Management System Operation and Maintenance	e13



TABLES

Table 1: Test Pit Observations	.2
Table 2: Rainfall Frequency Values	. 4
Table 3: Pre-Development Drainage Area Characteristics	
Table 4: Post-Development Drainage Area Characteristics	
Table 5: Basin Elevations (ft)	
Table 6: Design Point 1 Peak Runoff Rate, cfs	3.
Table 7: Design Point 2 Peak Runoff Rate, cfs	3.
Table 8: Design Point 3 Peak Runoff Rate, cfs	. (
Table 9: Design Point 4 Peak Runoff Rate, cfs	. (
Table 10: Water Quality Volume	

APPENDICES

HydroCAD Summary Report Water Quality Calculations NRCS Soils Map Appendix A Appendix B

Appendix C Appendix D Test Pit Logs

Appendix E Drainage Area Maps



Executive Summary

The goal of this analysis is to evaluate pre- and post-development stormwater conditions and develop a site design consistent with the Rhode Island Stormwater Management, Design and Installation Rules (250-RICR-150-10-8).

The Site is located at 2446 Victory Highway in Coventry, Rhode Island. The 117.3-acre parcel is identified as Assessor's Plat 304, Lot 27.1 and Lot 28. The property is privately owned and designated as land used for forestry, farming and related activities. The property also features an existing abandoned dwelling and two historical cemeteries totaling less than 2,000-SF in area. It consists of small open fields and vacant wooded lands surrounding the fields. There are large wetland complexes scattered throughout the northern and southern portions of the property, as well as a stream on the eastern side of the property outside of the limit of disturbance.

The Applicant proposes to construct a 4.37± MW direct current (DC) ground mounted photovoltaic solar array and corresponding electrical equipment, equipment pad, utility poles, fence, and stormwater basins. The project will be accessed by a crushed stone driveway to be constructed from Victory Highway. The proposed array occupies approximately 9.74± acres of the parcel and will be surrounded by a seven-foot-tall chain-link security fence, enclosing a total area of approximately 13.5± acres. A 6-inch clearance will be provided beneath the security fence to wildlife passage. The total Limits of Disturbance, including shade tree cutting, is 15.4± acres.

The results of the stormwater analysis indicate that the post-development conditions peak runoff rates generated by the 1, 10, and 100-year design storms will not exceed pre-development conditions. Post-development peak runoff rates have been mitigated using stormwater basins designed to infiltrate and/or provide some pretreatment of overland flow before exfiltrating or discharging to their respective Design Points. Stormwater attenuation, groundwater recharge, and water quality treatment will be provided by two stormwater basins and one sand filter providing infiltration and filtration. The 11 Minimum Stormwater Management Standards required by the Rhode Island Stormwater Management, Design and Installation Rules have been met, to the maximum extent practicable.



1.0 Purpose

The goal of this analysis is to evaluate pre- and post-development stormwater conditions and develop a Site design consistent with the Rhode Island Stormwater Management, Design and Installation Rules (250-RICR-150-10-8).

2.0 Existing Conditions

2.1 Existing Site Use

The Site is located at 2446 Victory Highway in Coventry, Rhode Island. The 117.3-acre parcel is identified as Assessor's Plat 304, Lot 27.1 and Lot 28. The two lots will be administratively combined after approval of the Site Plan by the Coventry Planning Commission. The Site is privately owned and designated as land used for forestry, farming and related activities. The Site is located in a Residential zone. The Site also features an existing abandoned residential dwelling and two historical cemeteries that total less than 2,000-SF in area. The Site consists of small open fields and vacant wooded lands surrounding the fields. There are large wetland complexes scattered throughout the northern and southern portions of the Site, as well as an unnamed stream located on the eastern side of the Site outside of the project Limit of Disturbance (LOD).

Outside of the wetlands, the majority of the slopes across the property range from approximately 0% to 25%. Ground elevations range from approximately 595 ft on the westernmost portion of the Site to 535 ft on the eastern portion of the LOD.

2.2 Hydrology

The western portion of the Site is located within the Flat River Reservoir sub-watershed (HUC 010900040602), while the eastern portion of the Site is located within the Big River sub-watershed (HUC 010900040601). According to RIDEM's online Environmental Resource Map, both sub-watersheds are listed as 303d Impaired Watersheds (RIDEM 2023a). The designed drainage basins will outlet into wetlands that converge to a tributary of the Quidneck Reservoir or the Nooseneck River. The Nooseneck River is listed as coldwater fisheries in 250-RICR-150-05-1 (Water Quality Regulations), Section 1.25.I.1 but was removed from the TMDL list in the 2022 Impaired Waters Report, dated December 2021. The Quidneck Reservoir is not listed as a coldwater fishery or TMDL.

2.3 Wetland Delineation

Wetlands were delineated by ESI in April and May 2022, and verified by TRC in November 2022. The Final Wetland report is dated January 2023.

The field survey resulted in the identification and delineation of several wetland complexes found throughout the Site, delineated by the flag series 'A', 'B', 'C', 'D', 'E', 'F', 'H' and 'I'.

Flag series A (A1-A74), F (F1-F47), and H (H1-H74) were flagged as three separate wetland areas but are a single hydrologically connected, deciduous forested swamp ≥10 acres. The



wetland area delineated by flag series F connects to the A flag series wetland offsite to the south. Both the A and F flag series direct flow towards the series H wetland, which flows offsite to the east.

Flag series B (B1-B4) is a small, isolated vernal pool located just to the northeast of series C.

Flag series C (C1-C4) is an isolated, excavated pit (vernal pool) to the southwest of series B with the top 0-1 inches at a color of 10YR 2/1, 1-6 inches at 2.5Y 4/3, and 6+ inches at 2.5Y 6/2.

Flag series D (D1-D50) is a deciduous forested swamp ≥10 acres in size, which extends offsite to the north. In the north half of the delineated area there are multiple wetland types within 50 feet of the wetland edge as the deciduous forested swamp transitions to a deciduous shrub swamp and then an emergent marsh. Per RIDEM, since these transitions happen within 50 feet of the wetland edge, the buffer zone in the areas between flags D1-D5 and flags D57-D66 will receive an additional 25 feet of buffer zone.

Flag series E (E1-E42) is a deciduous forested swamp between 1 and 10 acres in size, with very similar characteristics to flag series F. Due to its size being <10 acres, it is afforded a smaller buffer zone than the deciduous forested wetlands located onsite that are ≥10 acres in size

Flag series I (I1-I6) is a small, <1 acre isolated wetland to the northwest of series H.

2.4 Test Pits

Five test pits, identified TP-1 through TP-7, were observed by TRC on June 8 and June 14, 2023. The test pit logs are included in Appendix D and summarized in the following table.

ID	Estimated SHGW Depth (ft)	Restrictive Layer Depth	Design Soil Texture Class	Design Rawls Rate (in/hr)
TP-1	8.0	>8	Loamy Sand	2.41
TP-2	7.0	>8	Loamy Sand	2.41
TP-3	2.5	>6	Silt Loam	0.27
TP-5	3.4	>8	Loam	0.52
TP-6	3.3	>7.5	Loam	0.52
TP-7	3.0	>8	Sandy Loam	1.02

Table 1: Test Pit Observations

3.0 Proposed Development

The Applicant proposes to construct a 4.37± MW direct current (DC) ground mounted photovoltaic solar array and corresponding electrical equipment, equipment pad, utility poles, fence, and stormwater basins. The project will be accessed by a crushed stone driveway to be constructed from Victory Highway. The proposed array occupies approximately 9.74± acres of the parcel and will be surrounded by a seven-foot-tall chain-link security fence, enclosing a total



area of approximately 13.5± acres. A 6-inch clearance will be provided beneath the security fence to wildlife passage. The total LOD, including shade tree cutting, is 15.4± acres.

The ground within the fenced area and beneath the solar array will be cleared, grubbed and seeded with a low maintenance grass seed mix. Shade trees between the proposed fence and the solar array where no grading is proposed will be cut but not grubbed, leaving the existing ground cover intact. A restoration seed mix will be seeded over this area. The small open field area located on the north side of the site within the 40-foot vegetated buffer zone will be planted with native vegetation to maintain a visual buffer along that side of the Site consistent with the Coventry zoning ordinance. Seed mixes and landscape plantings are shown on the Landscape Plans.



4.0 Hydrologic and Hydraulic Analysis

4.1 Methodology

HydroCAD® software (developed by Applied Microcomputer Systems) was used to create a hydraulic and hydrologic model utilizing the methods prescribed in Soil Conservation Service (SCS) Technical Release No. 20 and SCS Technical Release No. 55. The HydroCAD® program calculates runoff based on rainfall and watershed characteristics and produces a runoff hydrograph (a runoff rate versus time curve). The stage-storage-discharge curves for a specific detention area are used to compute an outflow hydrograph by hydraulically routing an inflow hydrograph through a basin. This procedure calculates the relationship of the inflow hydrograph with the characteristics of the detention area to determine the outflow, stage, and storage capacity of the detention area for a given time during the specified storm event. All drainage analyses utilized Type III, 24-hour rainfall data from the Rhode Island Stormwater Management, Design and Installation Rules (250-RICR-150-10-8.6E) for Kent County. The rainfall frequency values used in this drainage analysis are listed in the table below.

Table 2: Rainfall Frequency Values

Frequency	1-Yr	10-Yr	100-Yr
Inches of Rainfall	2.7	4.8	8.7

Hydrographs were generated based on drainage area, hydrologic soil group, curve number (CN) values, times of concentration (Tc), and rainfall amount. The CN values for each drainage area were estimated by determining the composite value of the CN for the soil groups and ground cover mixture. Stormwater model simulations were performed for the 24-hour rainfall for the 1.2", 1, 10, and 100-year storm events using a Type III storm distribution.

The watershed characteristics for existing conditions, including flow patterns, were estimated based on topographic information determined by field survey and aerial photography. Refer to the HydroCAD calculations included in Appendix A.

4.2 Points Of Analysis

Four design points were evaluated based on existing drainage patterns and site characteristics. Each design point is summarized below and illustrated on the drainage area maps included in Appendix E.

- Design Point 1 (DP-1) represents a small <1 acre subcatchment on the northwestern side of the development that drains off-site.
- Design Point -2 (DP-2) represents a small < 1 acre subcatchment on the north-central portion of the development that drains off-site.
- Design Point 3 (DP-3) represents the drainage to wetlands delineated by flags 'D' and 'F' located to the east and south of the development.
- Design Point 4 (DP-4) represents the remaining drainage areas that discharge to wetlands delineated by flags 'A', 'B', 'C', 'E', 'H' and 'I'.



4.3 Pre-Development Drainage Areas

Design Points 1, 2 and 3 receive stormwater runoff from drainage areas 101, 106 and 102, respectively. Design Point 4 receives stormwater runoff from drainage areas 103, 104, 105 and 107. The drainage areas are summarized below and illustrated on drawing DA-1, included in Appendix E. Table 3 lists key characteristics of the hydrologic model for each drainage area.

These drainage areas are primarily wooded, with the exception of 102, which includes the existing residential gravel driveway and existing abandoned residential dwelling, and 103, which includes a portion of the existing residential gravel driveway.

The drainage areas contain varying soils classified as Hydrologic Soil Group (HSG) B & D soils. An NRCS Soils Map created from the Web Soil Survey that summarizes the varying HSG classifications of the soils can be found in Appendix C.

Drainage Area ID	Point of Analysis	Area (acres)	Curve Number	Time of Conc. (minutes)
101	DP-1	0.794	69	32.2
106	DP-2	0.946	68	19.1
102	DP-3	10.089	77	38.0
103		6.498	77	36.6
104	DD 4	2.650	77	21.3
105	DP-4	7.357	77	30.6
107		23.785	65	32.7

Table 3: Pre-Development Drainage Area Characteristics

4.4 Post-Development Drainage Areas

The post-development conditions stormwater runoff has been modeled as ten drainage areas that flow to the four Design Points. The drainage areas are summarized below and illustrated on drawing DA-2, included in Appendix E. Table 4 lists key characteristics of the hydrologic model for each drainage area.

- Drainage Area 201 drains to Design Point 1 (off-Site). It includes the northwestern portion of the Site, approximately 0.794 acres of undeveloped wooded area.
- Drainage Area 206U drains unattenuated off-Site to the north to Design Point 2. This area contains 0.5+/- acres of undeveloped woods and a portion of the proposed cleared area to develop the northern side of the solar array field.
- Drainage Area 206C drains to the Sand Filter before discharging to Design Point 2.
 This area contains a portion of the proposed cleared area to develop the northern side of the solar array field.
- Drainage Area 202 drains to directly to the northern wetland, Design Point 3. This area consists of 8+ acres of undeveloped woods and the new gravel access road leading to the solar array.



- Drainage Area 203 drains to Design Point 4. This area consists of 6+/- acres of undeveloped woods and a portion of the new gravel access road leading to the solar array.
- Drainage Area 204 drains to Design Point 4 and consists of mostly undeveloped wooded area on the southern side of the Site and a small portion of the new gravel access road leading to the solar array.
- Drainage Area 205U drains to Design Point 4 unattenuated. This area consists of the western portion of the proposed solar array and the eastern side of the proposed gravel access road.
- Drainage Area 205C drains to the West Basin before discharging to Design Point 4.
 This area consists primarily of the western portion of the proposed array.
- Drainage Area 207U drains to Design Point 4 unattenuated and consists of the central portion of the proposed array and some off-Site flow from the north.
- Drainage Area 207C includes the remainder of Drainage Area 207 that drains to the East Basin before discharging to Design Point 4. This area consists of the east-central portion of the solar array and a portion of off-Site flow from the north.

Time of Conc. **Drainage Area ID Design Point** Area (acres) **Curve Number** (minutes) 201 0.794 69 32.2 206U 69 24.2 0.847 2 206C 0.489 79 15.3 202 3 77 38.0 10.071 203 6.498 77 36.6 204 2.650 77 21.3 205U 5.302 78 29.4 4 205C 1.711 81 16.0 207U 16.815 65 32.9 207C 6.941 73 17.8

Table 4: Post-Development Drainage Area Characteristics

4.5 Proposed Stormwater Design

Stormwater Best Management Practices were designed in general accordance with the Stormwater Management, Design and Installation Rules to provide water quality treatment and attenuate peak flows from the 1, 10, and 100-year, Type III 24-hour design storms. They include level stone trenches, three basins, and a washed crushed stone access road surface.

• <u>Level stone trenches</u> – Level stone trenches are proposed to encourage sheet flow beneath the panel drip edges. The trenches are proposed beneath the panels where slopes exceed 8% and are not generally parallel with the array drip edge. They are 14 feet long by 2.5 feet wide with a reverse slope. The trenches will be installed parallel with the contours and spaced at intervals no longer than 100 feet.



- West Basin A 1 foot deep, 11,954 cubic foot infiltration basin will attenuate stormwater runoff from Drainage Area 205C and infiltrate the 1-yr and 10-yr design storms. A 50 foot wide spillway has been provided to accommodate the 100-yr design storm. Pretreatment will be provided by a pea gravel diaphragm.
- <u>Sand Filter</u> A 2 foot deep, 7,238 cubic foot sand filter will infiltrate stormwater runoff from Drainage Area 206C for the WQv and 1-yr and 10-yr design storms. A 10 foot wide spillway has been provided to accommodate the 100-yr design storm. Pre-treatment will be provided by a stone berm and shallow forebay area.
- <u>East Basin</u> A 3 foot deep, 73,563 cubic foot infiltration basin will attenuate stormwater runoff from Drainage Area 207C and infiltrate the 1-yr and 10-yr design storms. A 20 foot wide spillway has been provided to accommodate the 100-yr design storm. Pretreatment will be provided by a pea gravel diaphragm. A portion of the infiltration basin, where adjacent slopes exceed 15%, will be lined with an impervious liner to prevent undermining the adjacent steep slope.
- <u>Crushed Stone Road</u> The access road will be surfaced with a minimum of 6 inches of washed crushed stone and will be used infrequently; therefore, it is expected to remain pervious.

All basins have been designed to drain within 48 hours after the design storm ends.



4.6 Results

The results of the stormwater analysis indicate that the post-development conditions peak runoff rates generated by the design storms will not exceed pre-development conditions. The results are summarized in the tables below. Refer to the HydroCAD calculations provided in Appendix A for detailed results.

Table 5: Basin Elevations (ft)

	East Basin	West Basin	Sand Filter
Bottom	552.00	575.00	574.00
Spillway Crest	554.00	575.50	575.50
Top of Berm	555.00	576.00	576.00
1-yr Design Storm	552.43	575.10	573.34
10-yr Design Storm	553.74	575.51	574.64
100-yr Design Storm	554.43	575.66	575.56

Table 6: Design Point 1 Peak Runoff Rate, cfs

Design Storm	Pre-Dev	Post-Dev
1-yr	0.21	0.21
10-yr	0.89	0.89
100-yr	2.52	2.52

Table 7: Design Point 2 Peak Runoff Rate, cfs

Design Storm	Pre-Dev	Post-Dev
1-yr	0.27	0.25
10-yr	1.27	1.08
100-yr	3.67	3.05



Table 8: Design Point 3 Peak Runoff Rate, cfs

Design Storm	Pre-Dev	Post-Dev
1-yr	4.90	4.89
10-yr	14.64	14.61
100-yr	35.07	35.01

Table 9: Design Point 4 Peak Runoff Rate, cfs

Design Storm	Pre-Dev	Post-Dev
1-yr	12.09	10.10
10-yr	46.74	37.46
100-yr	127.81	118.31



5.0 Stormwater Management Standards

5.1 Minimum Standard 1: LID Site Planning and Design Strategies

LID site planning and design strategies must be used to the maximum extent practicable in order to reduce the generation of the water runoff volume for both new and redevelopment projects.

The following LID site planning and design strategies are proposed to the maximum extent practicable to reduce the volume of stormwater runoff generated:

- 1. The proposed array area has been sited to avoid steep slopes to the maximum extent practicable.
- 2. Site disturbance will be minimized by only grubbing areas required for construction. Stumps and existing groundcover will remain in areas of shade tree clearing.
- Grading has been minimized and is generally limited to construction of the crushed stone driveway, construction of BMPs and reducing the slope of areas too steep to accommodate the proposed racking.

5.2 Minimum Standard 2: Groundwater Recharge

Stormwater must be recharged within the same subwatershed to maintain baseflow at pre-development recharge levels to the maximum extent practicable in accordance with the requirements described in §§ 8.8(D) through (H) of this Part. Applicants may be required to provide a water budget analysis for proposed groundwater dewatering. Recharge volume is determined as a function of annual pre-development recharge for site-specific soils or surficial materials, average annual rainfall volume, and amount of impervious cover on a site. Recharge must occur in a manner that protects groundwater quality.

The groundwater recharge criterion (Re_v) is minimal as the only impervious surface proposed is a 1,539± square feet for the equipment pads. The equipment pads are located within Drainage Areas 205C and 206C. The total recharge volume is only 13± cubic feet. The West Basin and Sand Filter provide a total of 41,418 cubic feet of infiltration capacity.

The road will be constructed of washed crushed stone and will be used infrequently; therefore, it is expected to remain pervious. The long-term Operations and Maintenance Plan requires regrading the road as necessary to maintain its pervious characteristics if the water quality volume (one inch of runoff) does not infiltrate.

The recharge value was calculated using the formula given in 250-RICR-150-10-8.8D:

$$Re_v = 1" * F * I / 12$$

Where:

F = 0.10 (Hydrologic Soils Group D)



I = 1,539 sf (Proposed Impervious Area)

 $Re_v = 12.8 \text{ cf (Recharge Volume)}$

5.3 Minimum Standard 3: Water Quality

Stormwater runoff must be treated before discharge. The amount that must be treated from each rainfall event is known as the required water quality volume (WQv). The required WQv is calculated as described in §§ 8.9(E) through (J) of this Part and excludes LID credits allowed under §8.18 of this Part.

The WQv was calculated using the formula provided by 250-RICR-150-10-8.9(F):

$$WQ_v = 0.2" * I / 12$$

Where:

I = Disturbed area within the proposed solar array

WQ_v = Water Quality Volume (cubic feet)

The minimum WQv required is provided by two infiltration basins (East and West Basins) and an exfiltrating sand filter (Sand Filter). Refer to the detailed calculations provided in Appendix B.

Table 10: Water Quality Volume

Design Point	Required (cf)	Provided (cf)
2	599	5,258
4	8,927	51,898

5.4 Minimum Standard 4: Conveyance and Natural Channel Protection

Open drainage and pipe conveyance systems must be designed to provide adequate passage for flows leading to, from, and through stormwater management facilities for at least the peak flow from the 10-year, 24-hour Type III design storm event. Protection for natural channels downstream must be supplied by providing 24-hour extended detention of the 1-year, 24-hour Type III design storm event runoff volume.

For Design Points 1 and 2, the Channel Protection Volume criterion can be waived per 8.10.D.3, as the 1-year, post-development peak discharge without attenuation to each design point is less than 2 cfs. The unattenuated flow to Design Points 1 and 2 is 0.21 cfs and 0.27 cfs, respectively. For the remaining Design Points, the CPv criterion can be waived per 8.10.D.2, as the impervious cover is less than 1 acre.

The East Basin discharges to Wetland 'H' which eventually discharges to the Nooseneck River. The Nooseneck River is classified as a cold water fishery. The basin has been designed to infiltrate the 1-yr and 10-yr, 24-hour Type III design storm. Only larger storms discharge via spillways provided at the basin. Furthermore, the discharged stormwater flows into the unnamed stream and will have to travel over 500 feet prior to reaching the River.



5.5 Minimum Standard 5: Overbank Flood Protection

Downstream overbank flood protection must be provided by attenuating the post development peak discharge rate to the pre-development levels for the 10-year and 100-year, 24-hour Type III design storm events. In addition, designers must demonstrate that runoff from the site for storms up to the 100-year, 24-hour Type III design storm events actually reach proposed structural practices designed to meet this criterion.

Overbank flood protection will be provided by the proposed basins. The post-development peak discharge rates for the 10-year and 100-year design storms will be attenuated to or below predevelopment levels. Refer to Section 4.6.

5.6 Minimum Standard 6: Redevelopment and Infill Projects

Not Applicable - The proposed project is not a Redevelopment and Infill Project.

5.7 Minimum Standard 7: Pollution Prevention

All development sites require the use of source control and pollution prevention measures to minimize the impact that the land use may have on stormwater runoff quality. These measures shall be outlined in a stormwater pollution prevention plan.

The proposed project work will include low impact use of the project Site. No paving activities, solid waste generation, significant snow removal, or hazardous waste use is proposed. Low maintenance grasses that require little to no fertilization will be used.

5.8 Minimum Standard 8: Land Uses with Higher Potential Pollutant Loads

Stormwater discharges from land uses with higher potential pollutant loads (LUHPPLs) require the use of specific source control and pollution prevention measures and the specific stormwater BMPs approved for such use. Allowable BMPs for LUHPPLs are included in the Table in § 8.14(D) of this Part. Many LUHPPLs require additional special permits such as a RIPDES Multi-Sector General Permit, and sector-specific required BMPs are included in Section VI of the Multi-Sector General Permit.

Not Applicable -The proposed project is not a Land Use with Higher Potential Pollutant Loads.

5.9 Minimum Standard 9: Illicit Discharges

All illicit discharges to stormwater management systems are prohibited, including discharges from OWTS, and sub-drains and French drains near OWTS that do not meet the State's Rules Establishing Minimum Standards Relating to Location, Design, Construction and Maintenance of Onsite Wastewater Treatment Systems.

No illicit discharges have been identified or are proposed.



5.10 Minimum Standard 10: Construction Activity SESC and Pollution Prevention Control Measure

Soil Erosion and sedimentation control measures must be utilized during the construction phase as well as during any land disturbing activities.

A Soil Erosion and Sediment Control (SESC) Plan has been prepared.

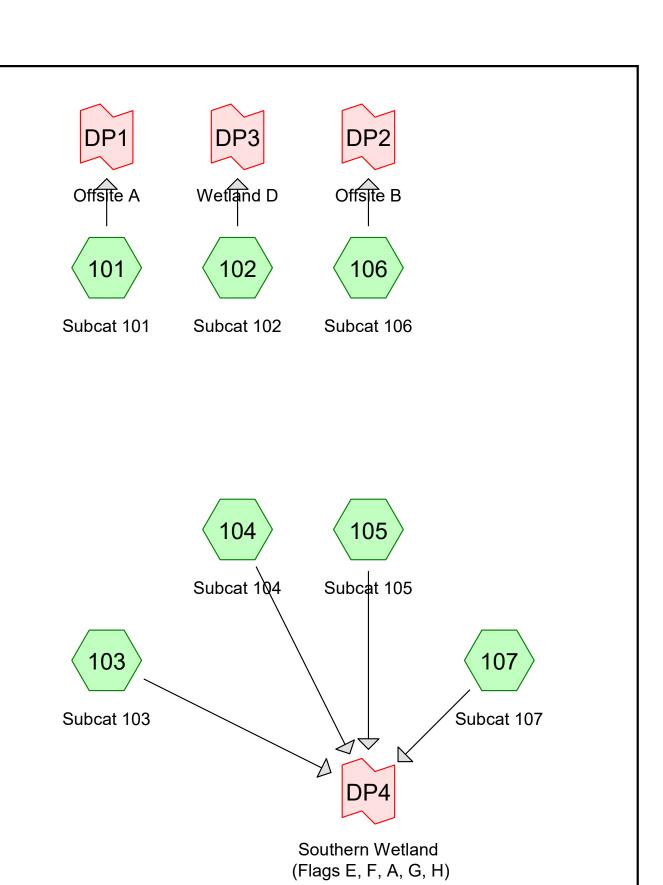
5.11 Minimum Standard 11: Stormwater Management System Operation and Maintenance

The stormwater management system, including all structural stormwater controls and conveyances, must have an Operation and Maintenance Plan to ensure that it continues to function as designed. The Operation and Maintenance Plan shall identify measures for implementing maintenance activities in a manner that minimizes stormwater runoff impacts.

A long-term Operation and Maintenance Plan has been prepared.



Attachment A: HydroCAD Summary Report











Prepared by TRC Companies, Printed 3/28/2024 HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

500563 Moo Cow Existing
Prepared by TRC Companies
HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Printed 3/28/2024 Page 2

Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	1-yr	Type III 24-hr		Default	24.00	1	2.70	2

Prepared by TRC Companies

HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 1-yr Rainfall=2.70" Printed 3/28/2024

Page 3

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment 101: Subcat 101 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=0.52"

Flow Length=379' Tc=32.2 min CN=69 Runoff=0.21 cfs 0.034 af

Subcatchment 102: Subcat 102 Runoff Area=10.089 ac 0.76% Impervious Runoff Depth=0.87"

Flow Length=870' Tc=38.0 min CN=77 Runoff=4.90 cfs 0.730 af

Subcatchment 103: Subcat 103 Runoff Area=6.498 ac 0.21% Impervious Runoff Depth=0.87"

Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=3.21 cfs 0.470 af

Subcatchment 104: Subcat 104 Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=0.87"

Flow Length=295' Tc=21.3 min CN=77 Runoff=1.66 cfs 0.192 af

Subcatchment 105: Subcat 105 Runoff Area=7.357 ac 0.00% Impervious Runoff Depth=0.87"

Flow Length=341' Tc=30.6 min CN=77 Runoff=3.95 cfs 0.533 af

Subcatchment 106: Subcat 106 Runoff Area=0.946 ac 0.00% Impervious Runoff Depth=0.48"

Flow Length=100' Slope=0.0255 '/' Tc=19.1 min CN=68 Runoff=0.27 cfs 0.038 af

Subcatchment 107: Subcat 107 Runoff Area=23.785 ac 0.00% Impervious Runoff Depth=0.38"

Flow Length=1,133' Tc=32.7 min CN=65 Runoff=3.90 cfs 0.745 af

Link DP1: Offsite A Inflow=0.21 cfs 0.034 af

Primary=0.21 cfs 0.034 af

Link DP2: Offsite B Inflow=0.27 cfs 0.038 af

Primary=0.27 cfs 0.038 af

Link DP3: Wetland D Inflow=4.90 cfs 0.730 af

Primary=4.90 cfs 0.730 af

Link DP4: Southern Wetland (Flags E, F, A, G, H) Inflow=12.09 cfs 1.940 af

Primary=12.09 cfs 1.940 af

Total Runoff Area = 52.119 ac Runoff Volume = 2.742 af Average Runoff Depth = 0.63" 99.83% Pervious = 52.029 ac 0.17% Impervious = 0.090 ac

HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 101: Subcat 101

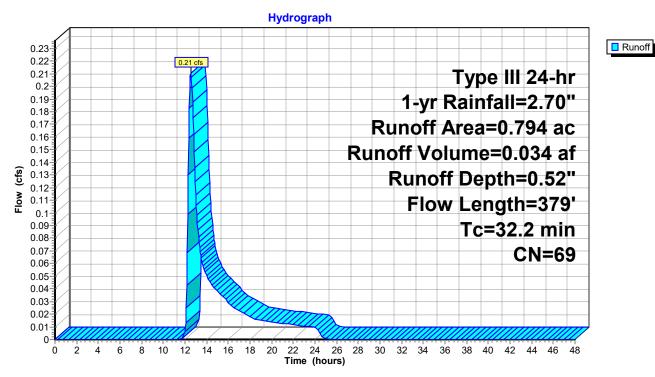
Runoff = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af, Depth= 0.52"

Routed to Link DP1: Offsite A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

_	Area	(ac) C	N Desc	cription		
	_			ds, Good,		
_	0.	521 7	77 Woo	Woods, Good, HSG D		
	0.	794 6	89 Weig	ghted Aver	age	
	0.	794	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	27.0	100	0.0108	0.06		Sheet Flow, woods
						Woods: Light underbrush n= 0.400 P2= 3.32"
	5.2	279	0.0314	0.89		Shallow Concentrated Flow, woods
						Woodland Kv= 5.0 fps
_	32.2	379	Total			·

Subcatchment 101: Subcat 101



Summary for Subcatchment 102: Subcat 102

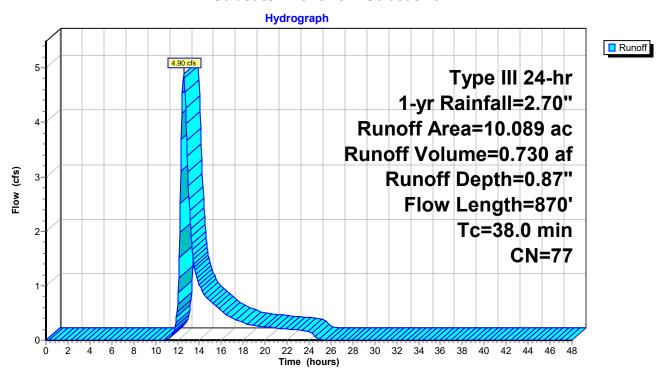
Runoff = 4.90 cfs @ 12.57 hrs, Volume= 0.730 af, Depth= 0.87"

Routed to Link DP3: Wetland D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area	(ac) C	N De	scription					
1.	158	80 >7	5% Grass c	over, Good	, HSG D			
0.	074	96 Gra	avel surface	, HSG D				
0.	076		HSG D					
0.413 55 Woods, Good, HSG B								
8.	367	77 Wc	ods, Good,	HSG D				
10.	089	77 We	ighted Avei	rage				
10.	013	99.	24% Pervio	us Area				
0.	076	0.7	6% Impervi	ous Area				
0.	076	100	0.00% Unco	nnected				
Тс	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
19.4	100	0.0246	0.09		Sheet Flow, sheet			
					Woods: Light underbrush n= 0.400 P2= 3.32"			
18.6	770	0.0190	0.69		Shallow Concentrated Flow, woods			
					Woodland Kv= 5.0 fps			
38.0	870	Total						

Subcatchment 102: Subcat 102



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 103: Subcat 103

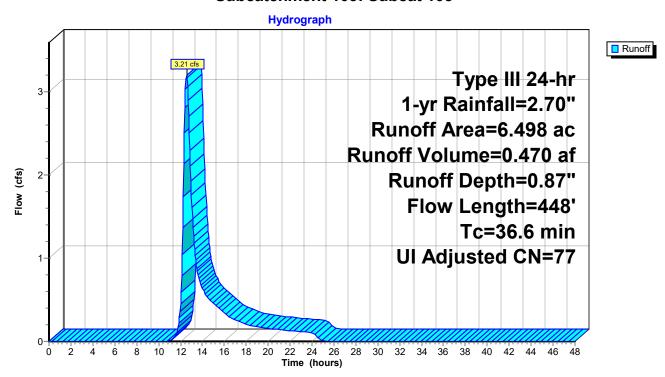
Runoff = 3.21 cfs @ 12.55 hrs, Volume= 0.470 af, Depth= 0.87"

Routed to Link DP4: Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area ((ac)	C١	N Adj	Descript	tion				
	0.	346	80)	>75% G	rass cover	, Good, HSG D			
	0.	103	96	3	Gravel s	Gravel surface, HSG D				
	0.	013	98	3	Unconn	nected pavement, HSG D				
	6.	035	77	7	Woods,	Good, HS0	G D			
	6.	498	78	3 77	Weighte	ed Average	, UI Adjusted			
	6.	484			99.79%	Pervious A	ırea			
	0.	013			0.21% Impervious Area					
	0.013				100.00% Unconnected					
	Tc	Lengtl	h	Slope	Velocity	Capacity	Description			
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)				
	27.8	10	0	0.0100	0.06		Sheet Flow, sheet			
							Woods: Light underbrush n= 0.400 P2= 3.32"			
	8.8	348	8	0.0172	0.66		Shallow Concentrated Flow, woods			
_							Woodland Kv= 5.0 fps			
	36.6	448	8	Total						

Subcatchment 103: Subcat 103



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 104: Subcat 104

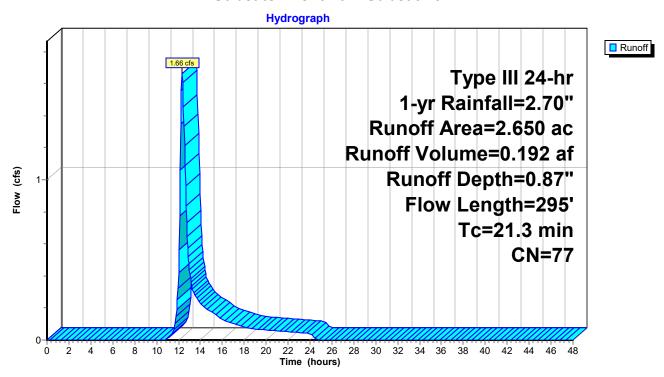
Runoff = 1.66 cfs @ 12.32 hrs, Volume = 0.192 af, Depth = 0.87"

Routed to Link DP4: Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac) C	N Desc	cription		
	2.	650 7	77 Woo	ds, Good,	HSG D	
_	2.650		100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	16.9	100	0.0350	0.10	, ,	Sheet Flow, sheet Woods: Light underbrush n= 0.400 P2= 3.32"
	4.4	195	0.0219	0.74		Shallow Concentrated Flow, woods Woodland Kv= 5.0 fps
	21.3	295	Total			

Subcatchment 104: Subcat 104



Summary for Subcatchment 105: Subcat 105

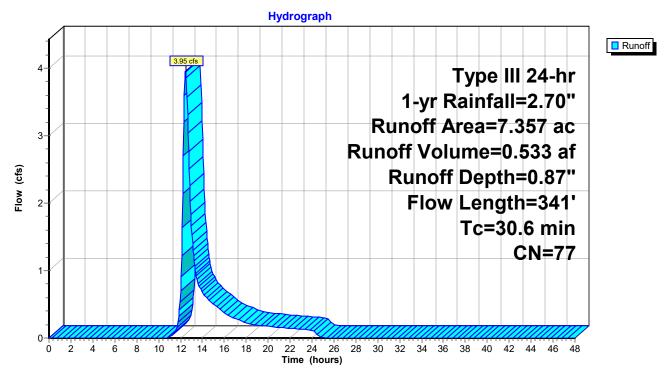
Runoff = 3.95 cfs @ 12.47 hrs, Volume= 0.533 af, Depth= 0.87"

Routed to Link DP4: Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

_	Area	(ac) C	N Desc	cription		
Ī	7.	357 7	77 Woo	ds, Good,	HSG D	
-	7.	357	100.00% Pervious Area			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	23.8	100	0.0147	0.07	, ,	Sheet Flow, woods
	6.8	241	0.0138	0.59		Woods: Light underbrush n= 0.400 P2= 3.32" Shallow Concentrated Flow, woods Woodland Kv= 5.0 fps
	30.6	341	Total			

Subcatchment 105: Subcat 105



Summary for Subcatchment 106: Subcat 106

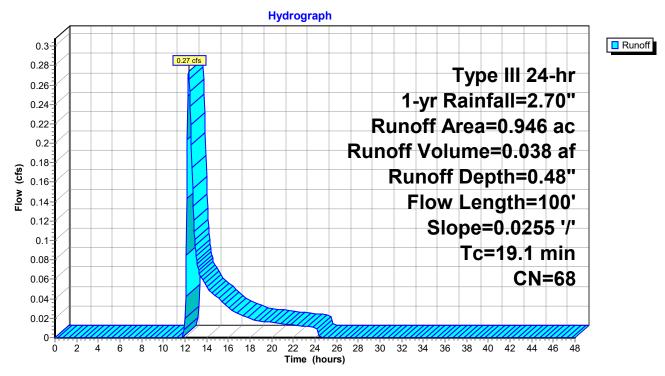
0.038 af, Depth= 0.48" Runoff 0.27 cfs @ 12.34 hrs, Volume=

Routed to Link DP2: Offsite B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area	(ac)	CN	Desc	cription				
0	.394	55	Woo	ds, Good,	HSG B			
0	.552	77	Woo	ds, Good,	HSG D			
0	.946	68	Weig	ghted Aver	age			
0	.946		100.0	00% Pervi	ous Area			
т.	ا محمد	L (Nama.	\/alaaita	Compoitu	Description		
Tc	Lengt		Slope	Velocity	Capacity	Description		
(min)	(fee	ι)	(ft/ft)	(ft/sec)	(cfs)			
19.1	10	0 0.	0255	0.09		Sheet Flow, woods		
						Woods: Light underbrush	n = 0.400	P2= 3.32"

Subcatchment 106: Subcat 106



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 107: Subcat 107

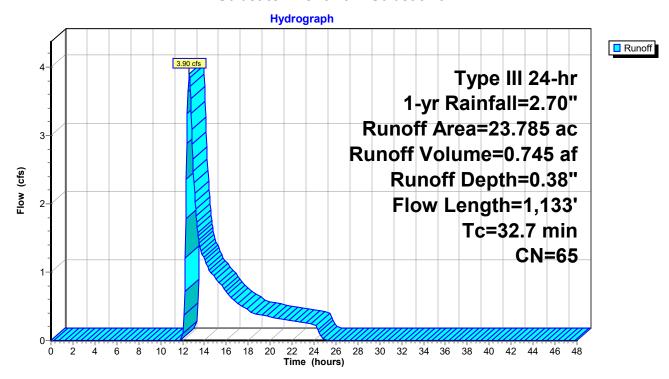
Runoff = 3.90 cfs @ 12.60 hrs, Volume= 0.745 af, Depth= 0.38"

Routed to Link DP4: Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area (ac) CN Description						
12.690 55 Woods, Good, HSG B						
_	11.	095 7	77 Woo	ds, Good,	HSG D	
23.785 65 Weighted Average						
	23.	785	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	17.5	100	0.0318	0.10		Sheet Flow, woods
						Woods: Light underbrush n= 0.400 P2= 3.32"
	15.2	1,033	0.0511	1.13		Shallow Concentrated Flow, woods
						Woodland Kv= 5.0 fps
_	32 7	1 133	Total	•	•	

Subcatchment 107: Subcat 107



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Link DP1: Offsite A

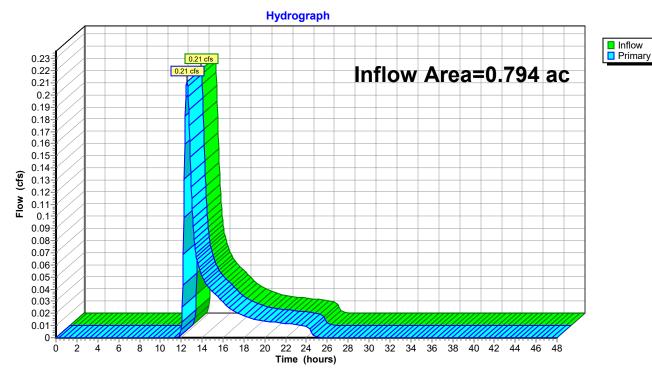
Inflow Area = 0.794 ac, 0.00% Impervious, Inflow Depth = 0.52" for 1-yr event

Inflow = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af

Primary = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP1: Offsite A



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 12

Summary for Link DP2: Offsite B

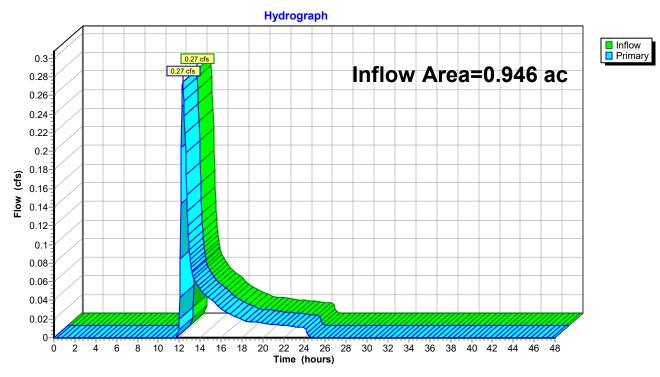
Inflow Area = 0.946 ac, 0.00% Impervious, Inflow Depth = 0.48" for 1-yr event

Inflow = 0.27 cfs @ 12.34 hrs, Volume= 0.038 af

Primary = 0.27 cfs @ 12.34 hrs, Volume= 0.038 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP2: Offsite B



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 13

Summary for Link DP3: Wetland D

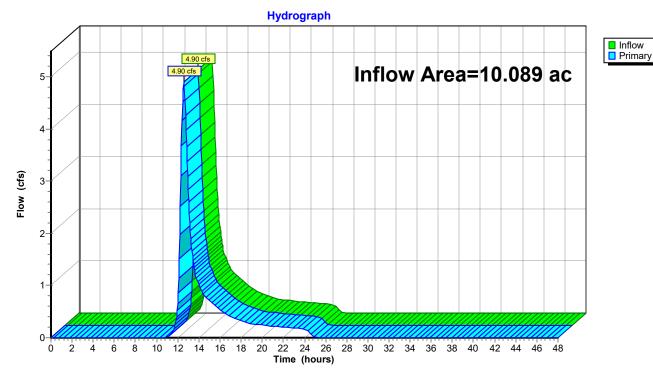
Inflow Area = 10.089 ac, 0.76% Impervious, Inflow Depth = 0.87" for 1-yr event

Inflow = 4.90 cfs @ 12.57 hrs, Volume= 0.730 af

Primary = 4.90 cfs @ 12.57 hrs, Volume= 0.730 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP3: Wetland D



HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Link DP4: Southern Wetland (Flags E, F, A, G, H)

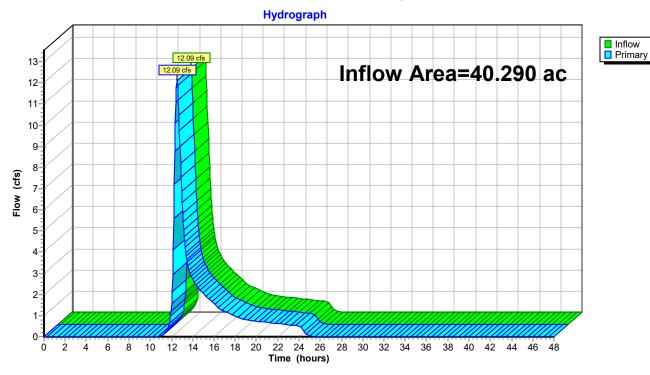
Inflow Area = 40.290 ac, 0.03% Impervious, Inflow Depth = 0.58" for 1-yr event

Inflow = 12.09 cfs @ 12.52 hrs, Volume= 1.940 af

Primary = 12.09 cfs @ 12.52 hrs, Volume= 1.940 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP4: Southern Wetland (Flags E, F, A, G, H)



Prepared by TRC Companies

HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 1.2" Rainfall=1.20" Printed 3/28/2024

Page 15

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment 101: Subcat 101 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=0.02"

Flow Length=379' Tc=32.2 min CN=69 Runoff=0.00 cfs 0.001 af

Subcatchment 102: Subcat 102 Runoff Area=10.089 ac 0.76% Impervious Runoff Depth=0.10"

Flow Length=870' Tc=38.0 min CN=77 Runoff=0.29 cfs 0.085 af

Subcatchment 103: Subcat 103 Runoff Area=6.498 ac 0.21% Impervious Runoff Depth=0.10"

Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=0.19 cfs 0.055 af

Subcatchment 104: Subcat 104 Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=0.10"

Flow Length=295' Tc=21.3 min CN=77 Runoff=0.10 cfs 0.022 af

Subcatchment 105: Subcat 105 Runoff Area=7.357 ac 0.00% Impervious Runoff Depth=0.10"

Flow Length=341' Tc=30.6 min CN=77 Runoff=0.24 cfs 0.062 af

Subcatchment 106: Subcat 106 Runoff Area=0.946 ac 0.00% Impervious Runoff Depth=0.01"

Flow Length=100' Slope=0.0255 '/' Tc=19.1 min CN=68 Runoff=0.00 cfs 0.001 af

Subcatchment 107: Subcat 107 Runoff Area=23.785 ac 0.00% Impervious Runoff Depth=0.00"

Flow Length=1,133' Tc=32.7 min CN=65 Runoff=0.01 cfs 0.005 af

Link DP1: Offsite A Inflow=0.00 cfs 0.001 af

Primary=0.00 cfs 0.001 af

Link DP2: Offsite B Inflow=0.00 cfs 0.001 af

Primary=0.00 cfs 0.001 af

Link DP3: Wetland D Inflow=0.29 cfs 0.085 af

Primary=0.29 cfs 0.085 af

Link DP4: Southern Wetland (Flags E, F, A, G, H) Inflow=0.51 cfs 0.145 af

Primary=0.51 cfs 0.145 af

Prepared by TRC Companies

HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 10-yr Rainfall=4.80" Printed 3/28/2024

Page 16

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment 101: Subcat 101 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=1.81"

Flow Length=379' Tc=32.2 min CN=69 Runoff=0.89 cfs 0.120 af

Subcatchment 102: Subcat 102 Runoff Area=10.089 ac 0.76% Impervious Runoff Depth=2.46"

Flow Length=870' Tc=38.0 min CN=77 Runoff=14.64 cfs 2.065 af

Subcatchment 103: Subcat 103 Runoff Area=6.498 ac 0.21% Impervious Runoff Depth=2.46"

Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=9.60 cfs 1.330 af

Subcatchment 104: Subcat 104 Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=2.46"

Flow Length=295' Tc=21.3 min CN=77 Runoff=4.99 cfs 0.543 af

Subcatchment 105: Subcat 105 Runoff Area=7.357 ac 0.00% Impervious Runoff Depth=2.46"

Flow Length=341' Tc=30.6 min CN=77 Runoff=11.82 cfs 1.506 af

Subcatchment 106: Subcat 106 Runoff Area=0.946 ac 0.00% Impervious Runoff Depth=1.74"

Flow Length=100' Slope=0.0255 '/' Tc=19.1 min CN=68 Runoff=1.27 cfs 0.137 af

Subcatchment 107: Subcat 107 Runoff Area=23.785 ac 0.00% Impervious Runoff Depth=1.52"

Flow Length=1,133' Tc=32.7 min CN=65 Runoff=21.67 cfs 3.017 af

Link DP1: Offsite A Inflow=0.89 cfs 0.120 af

Primary=0.89 cfs 0.120 af

Link DP2: Offsite B Inflow=1.27 cfs 0.137 af

Primary=1.27 cfs 0.137 af

Link DP3: Wetland D Inflow=14.64 cfs 2.065 af

Primary=14.64 cfs 2.065 af

Link DP4: Southern Wetland (Flags E, F, A, G, H) Inflow=46.74 cfs 6.395 af

Primary=46.74 cfs 6.395 af

Prepared by TRC Companies
HydroCAD® 10.20-4a s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 100-yr Rainfall=8.70" Printed 3/28/2024

Page 17

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment 101: Subcat 101 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=4.95"

Flow Length=379' Tc=32.2 min CN=69 Runoff=2.52 cfs 0.328 af

Subcatchment 102: Subcat 102 Runoff Area=10.089 ac 0.76% Impervious Runoff Depth=5.92"

Flow Length=870' Tc=38.0 min CN=77 Runoff=35.07 cfs 4.977 af

Subcatchment 103: Subcat 103 Runoff Area=6.498 ac 0.21% Impervious Runoff Depth=5.92"

Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=23.02 cfs 3.206 af

Subcatchment 104: Subcat 104 Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=5.92"

Flow Length=295' Tc=21.3 min CN=77 Runoff=11.95 cfs 1.308 af

Subcatchment 105: Subcat 105 Runoff Area=7.357 ac 0.00% Impervious Runoff Depth=5.92"

Flow Length=341' Tc=30.6 min CN=77 Runoff=28.37 cfs 3.629 af

Subcatchment 106: Subcat 106 Runoff Area=0.946 ac 0.00% Impervious Runoff Depth=4.83"

Flow Length=100' Slope=0.0255 '/' Tc=19.1 min CN=68 Runoff=3.67 cfs 0.381 af

Subcatchment 107: Subcat 107 Runoff Area=23.785 ac 0.00% Impervious Runoff Depth=4.47"

Flow Length=1,133' Tc=32.7 min CN=65 Runoff=67.49 cfs 8.855 af

Link DP1: Offsite A Inflow=2.52 cfs 0.328 af

Primary=2.52 cfs 0.328 af

Link DP2: Offsite B Inflow=3.67 cfs 0.381 af

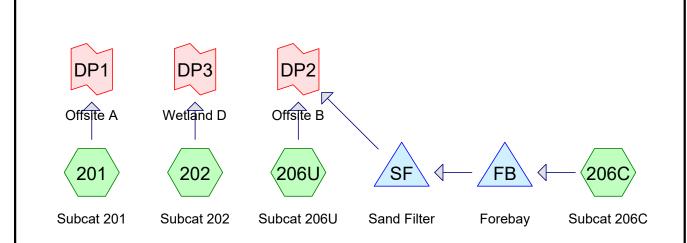
Primary=3.67 cfs 0.381 af

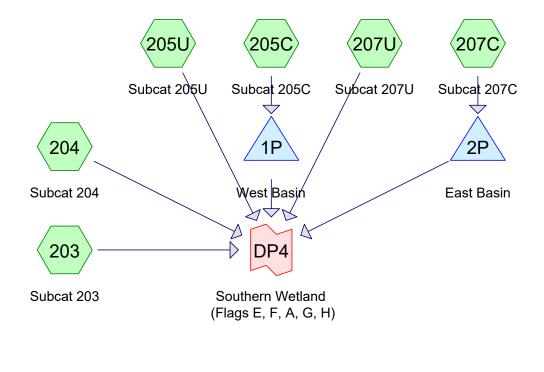
Link DP3: Wetland D Inflow=35.07 cfs 4.977 af

Primary=35.07 cfs 4.977 af

Link DP4: Southern Wetland (Flags E, F, A, G, H) Inflow=127.81 cfs 16.998 af

Primary=127.81 cfs 16.998 af













Printed 4/15/2024

Page 2

Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	1-yr	Type III 24-hr		Default	24.00	1	2.70	2

Prepared by TRC Companies

Link DP2: Offsite B

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 1-yr Rainfall=2.70" Printed 4/15/2024

Inflow=0.25 cfs 0.036 af Primary=0.25 cfs 0.036 af

Page 3

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Readiniedin	ig by Maskinga	and during mounds. I and routing by otor ma mounds
Subcatchment201: Subc	cat 201	Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=0.52" Flow Length=379' Tc=32.2 min CN=69 Runoff=0.21 cfs 0.034 af
Subcatchment202: Subc	cat 202	Runoff Area=10.071 ac 0.00% Impervious Runoff Depth=0.87" Flow Length=870' Tc=38.0 min CN=77 Runoff=4.89 cfs 0.729 af
Subcatchment203: Subc		Runoff Area=6.498 ac 0.14% Impervious Runoff Depth=0.87" gth=448' Tc=36.6 min UI Adjusted CN=77 Runoff=3.21 cfs 0.470 af
Subcatchment204: Subc	cat 204	Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=0.87" Flow Length=295' Tc=21.3 min CN=77 Runoff=1.66 cfs 0.192 af
Subcatchment205C: Su	bcat 205C	Runoff Area=1.711 ac 1.03% Impervious Runoff Depth=1.09" Flow Length=224' Tc=16.0 min CN=81 Runoff=1.56 cfs 0.155 af
Subcatchment205U: Su	bcat 205U	Runoff Area=5.302 ac 0.02% Impervious Runoff Depth=0.92" Flow Length=283' Tc=29.4 min CN=78 Runoff=3.11 cfs 0.407 af
Subcatchment206C: Su	bcat 206C	Runoff Area=0.489 ac 3.41% Impervious Runoff Depth=0.97" Flow Length=185' Tc=15.3 min CN=79 Runoff=0.40 cfs 0.040 af
Subcatchment206U: Su	bcat 206U	Runoff Area=0.847 ac 0.00% Impervious Runoff Depth=0.52" Flow Length=128' Tc=24.2 min CN=69 Runoff=0.25 cfs 0.036 af
Subcatchment207C: Su	bcat 207C	Runoff Area=6.941 ac 0.00% Impervious Runoff Depth=0.68" Flow Length=761' Tc=17.8 min CN=73 Runoff=3.41 cfs 0.393 af
Subcatchment207U: Su	bcat 207U	Runoff Area=16.815 ac 0.00% Impervious Runoff Depth=0.38" Flow Length=1,133' Tc=32.9 min CN=65 Runoff=2.75 cfs 0.527 af
Pond 1P: West Basin	Discarded=0.6	Peak Elev=575.10' Storage=1,180 cf Inflow=1.56 cfs 0.155 af 3 cfs 0.155 af Primary=0.00 cfs 0.000 af Outflow=0.63 cfs 0.155 af
Pond 2P: East Basin	Discarded=0.2	Peak Elev=552.43' Storage=8,890 cf Inflow=3.41 cfs 0.393 af 26 cfs 0.393 af Primary=0.00 cfs 0.000 af Outflow=0.26 cfs 0.393 af
Pond FB: Forebay		Peak Elev=574.54' Storage=203 cf Inflow=0.40 cfs 0.040 af Outflow=0.42 cfs 0.035 af
Pond SF: Sand Filter	Discarded=0.0	Peak Elev=573.34' Storage=566 cf Inflow=0.42 cfs 0.035 af 95 cfs 0.035 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.035 af
Link DP1: Offsite A		Inflow=0.21 cfs 0.034 af Primary=0.21 cfs 0.034 af

Type III 24-hr 1-yr Rainfall=2.70"

Prepared by TRC Companies
HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Printed 4/15/2024

Page 4

Link DP3: Wetland D Inflow=4.89 cfs 0.729 af Primary=4.89 cfs 0.729 af

Link DP4: Southern Wetland (Flags E, F, A, G, H)

Inflow=10.10 cfs 1.596 af
Primary=10.10 cfs 1.596 af

Total Runoff Area = 52.119 ac Runoff Volume = 2.983 af Average Runoff Depth = 0.69" 99.92% Pervious = 52.075 ac 0.08% Impervious = 0.044 ac

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 201: Subcat 201

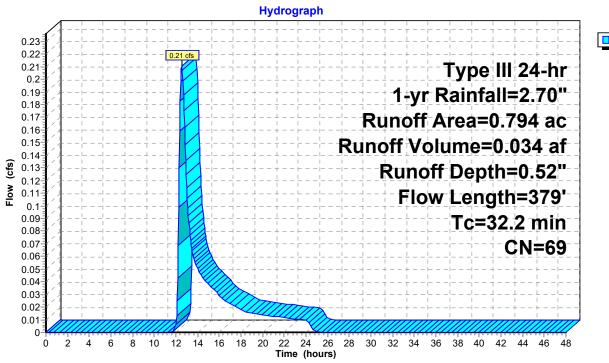
Runoff = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af, Depth= 0.52"

Routed to Link DP1: Offsite A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac) C	N Desc	cription						
	0.	273 5	55 Woo	ds, Good,	HSG B					
0.521 77 Woods, Good, HSG D										
	0.794 69 Weighted Average									
	0.	794	100.	00% Pervi	ous Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	27.0	100	0.0108	0.06		Sheet Flow, woods				
	5.2	279	0.0314	0.89		Woods: Light underbrush n= 0.400 P2= 3.32" Shallow Concentrated Flow, woods Woodland Kv= 5.0 fps				
	32.2	379	Total							

Subcatchment 201: Subcat 201





HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 202: Subcat 202

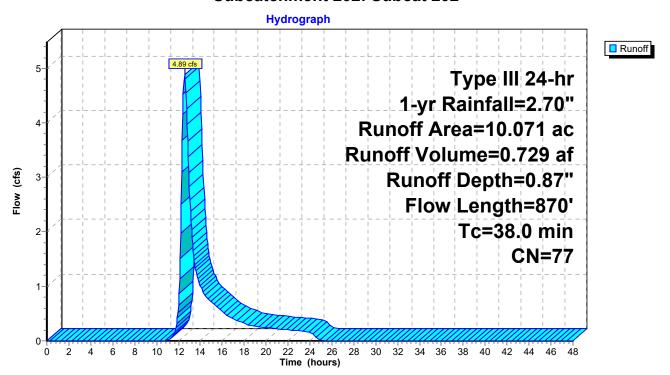
Runoff = 4.89 cfs @ 12.57 hrs, Volume= 0.729 af, Depth= 0.87"

Routed to Link DP3: Wetland D

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac)	CN	Desc	cription			
	0.	929	80	>75%	√ Grass co	over, Good	, HSG D	
	0.524 96			Grav	el surface	, HSG D		
	0.	413	55	Woo	ds, Good,	HSG B		
8.206 77 Woods, Good, HSG D								
	10.071 77 Weighted Average							
	10.	071		100.	00% Pervi	ous Area		
	Тс	Lengtl	h	Slope	Velocity	Capacity	Description	
	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)		
	19.4	100	0 0	0.0246	0.09		Sheet Flow, woods	
							Woods: Light underbrush n= 0.400 P2= 3.32"	
	18.6	770	0 0	0.0190	0.69		Shallow Concentrated Flow, woods	
							Woodland Kv= 5.0 fps	
	38.0	870	0 T	otal	·			

Subcatchment 202: Subcat 202



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

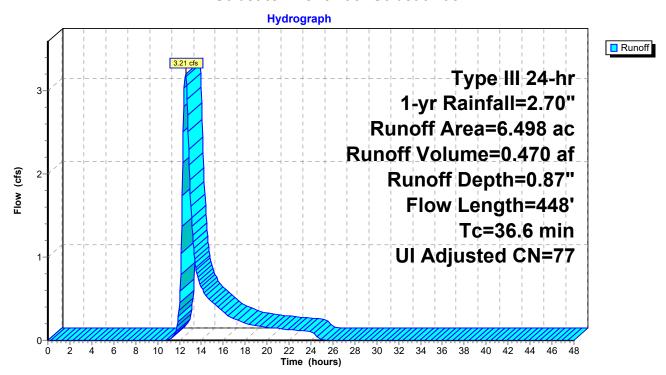
Summary for Subcatchment 203: Subcat 203

Runoff = 3.21 cfs @ 12.55 hrs, Volume= 0.470 af, Depth= 0.87" Routed to Link DP4 : Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area ((ac) (CN Adj	Descrip	Description						
	0.3	302	80	>75% G	rass cover	, Good, HSG D					
	0.	118	96	Gravels	Gravel surface, HSG D						
	0.0	009	98	Unconn	Unconnected pavement, HSG D						
	6.069 77 Woods, Good, HSG D										
	6.498 78 77 Weighted Average, UI Adjusted										
	6.4	489		99.86%	Pervious A	rea					
	0.0	009		0.14% I	0.14% Impervious Area						
	0.009			100.009	6 Unconne	cted					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	27.8	100	0.0100	0.06		Sheet Flow, woods					
						Woods: Light underbrush n= 0.400 P2= 3.32"					
	8.8	348	0.0172	0.66		Shallow Concentrated Flow, woods					
						Woodland Kv= 5.0 fps					
	36.6	448	Total								

Subcatchment 203: Subcat 203



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

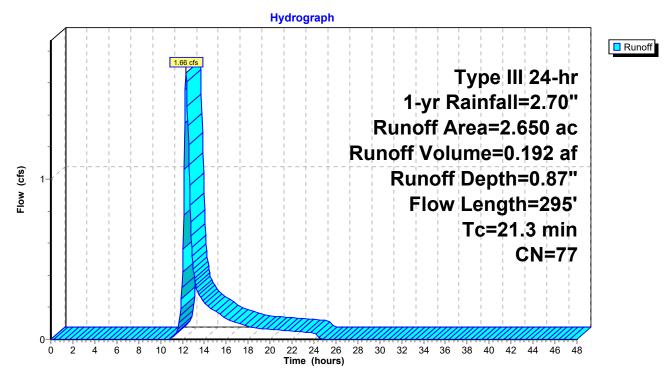
Summary for Subcatchment 204: Subcat 204

Runoff = 1.66 cfs @ 12.32 hrs, Volume= 0.192 af, Depth= 0.87" Routed to Link DP4 : Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac) C	N Des	cription						
	0.	007	96 Grav	el surface	, HSG D					
2.643 77 Woods, Good, HSG D										
	2.650 77 Weighted Average									
	2.	650	100.	00% Pervi	ous Area					
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	16.9	100	0.0350	0.10		Sheet Flow, woods				
						Woods: Light underbrush n= 0.400 P2= 3.32"				
	4.4	195	0.0219	0.74		Shallow Concentrated Flow, woods				
						Woodland Kv= 5.0 fps				
	21.3	295	Total							

Subcatchment 204: Subcat 204



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 205C: Subcat 205C

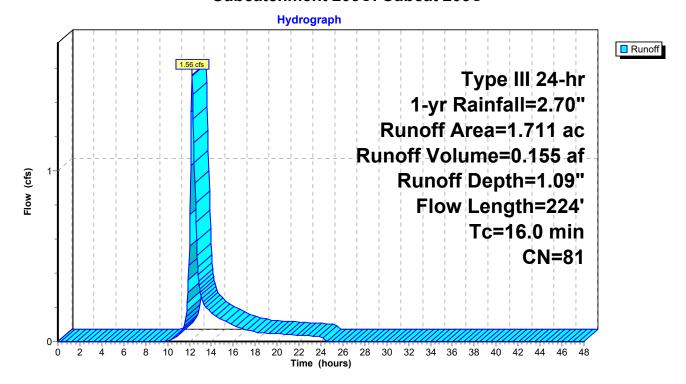
Runoff = 1.56 cfs @ 12.23 hrs, Volume= 0.155 af, Depth= 1.09"

Routed to Pond 1P: West Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac)	CN	Desc	cription					
	1.	647	80	>75%	√ Grass co	over, Good	, HSG D			
	0.	047	96	Grav	el surface	, HSG D				
0.018 98 Unconnected pavement, HSG D							HSG D			
	1.711 81 Weighted Average									
	1.694 98.97% Pervious Area									
	0.	018		1.039	1.03% Impervious Area					
	0.	018		100.0	100.00% Unconnected					
	Tc (min)	Lengti (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	12.7	100	0.	0100	0.13		Sheet Flow, grass			
	3.3	124	4 0.	0800	0.63		Grass: Short n= 0.150 P2= 3.32" Shallow Concentrated Flow, grass Short Grass Pasture Kv= 7.0 fps			
	16.0	22	4 To	otal						

Subcatchment 205C: Subcat 205C



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 10

Summary for Subcatchment 205U: Subcat 205U

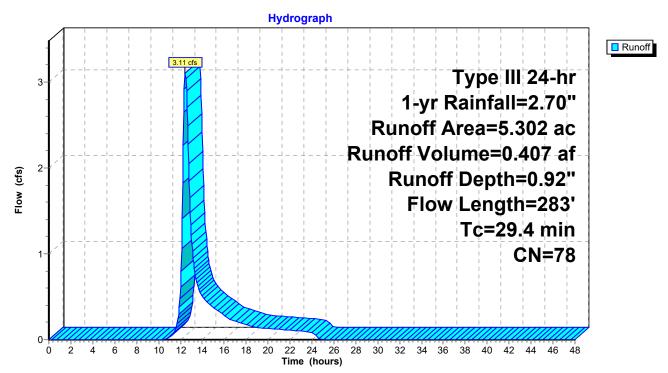
Runoff = 3.11 cfs @ 12.44 hrs, Volume= 0.407 af, Depth= 0.92" Routed to Link DP4 : Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area	(ac) C	N Desc	cription							
0.	.898 8	30 >759	% Grass co	over, Good	, HSG D					
0.	.113		el surface	•	,					
0.	HSG D									
4.290 77 Woods, Good, HSG D										
5.	5.302 78 Weighted Average									
5.	5.301 99.98% Pervious Area									
0.	.001	0.02	% Impervi	ous Area						
0.	.001	100.	00% Ünco	nnected						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
19.8	100	0.0234	0.08		Sheet Flow, woods					
					Woods: Light underbrush n= 0.400 P2= 3.32"					
2.8	99	0.0137	0.59		Shallow Concentrated Flow, woods					
					Woodland Kv= 5.0 fps					
0.6	25	0.0020	0.72		Shallow Concentrated Flow, road					
					Unpaved Kv= 16.1 fps					
6.2	59	0.0010	0.16		Shallow Concentrated Flow, woods					
					Woodland Kv= 5.0 fps					
29.4	283	Total								

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Subcatchment 205U: Subcat 205U



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Subcatchment 206C: Subcat 206C

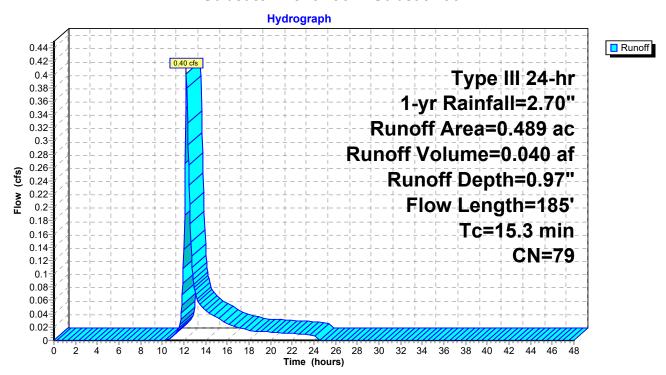
Runoff = 0.40 cfs @ 12.22 hrs, Volume= 0.040 af, Depth= 0.97"

Routed to Pond FB: Forebay

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac)	CN	Desc	cription				
	0.030 61 >75% Grass cover, Good, HSG B								
	0.442 80 >75% Grass cover, Good, HSG D								
0.017 98 Unconnected pavement, HSG D									
	0.	000	77	Woo	ds, Good,	HSG D			
	0.489 79 Weighted Average								
	0.	473		96.5	9% Pervio	us Area			
	0.	017		3.41	% Impervi	ous Area			
	0.	017		100.	00% Unco	nnected			
	Тс	Length	າ S	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	12.7	100	0.	0100	0.13		Sheet Flow, grass		
							Grass: Short n= 0.150 P2= 3.32"		
	2.6	85	5 0.	0060	0.54		Shallow Concentrated Flow, grass		
							Short Grass Pasture Kv= 7.0 fps		
	15.3	185	5 To	otal					

Subcatchment 206C: Subcat 206C



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

<u>Page 13</u>

Summary for Subcatchment 206U: Subcat 206U

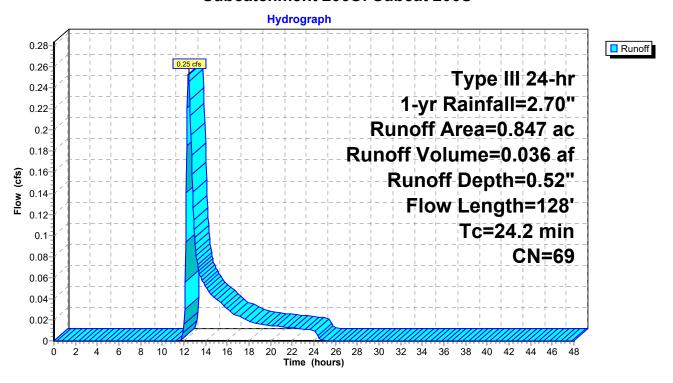
Runoff = 0.25 cfs @ 12.42 hrs, Volume= 0.036 af, Depth= 0.52"

Routed to Link DP2: Offsite B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area (ac) C	N Desc	cription						
0.148 61 >75% Grass cover, Good, HSG B									
0.203 80 >75% Grass cover, Good, HSG D									
0.2	0.211 55 Woods, Good, HSG B								
0.2	0.286 77 Woods, Good, HSG D								
3.0	0.847 69 Weighted Average								
0.8	347	100.	00% Pervi	ous Area					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
23.6	100	0.0151	0.07		Sheet Flow, woods				
					Woods: Light underbrush n= 0.400 P2= 3.32"				
0.6	28	0.0271	0.82		Shallow Concentrated Flow, woods				
					Woodland Kv= 5.0 fps				
24.2	128	Total	•						

Subcatchment 206U: Subcat 206U



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 14

Summary for Subcatchment 207C: Subcat 207C

Runoff = 3.41 cfs @ 12.28 hrs, Volume= 0.393 af, Depth= 0.68"

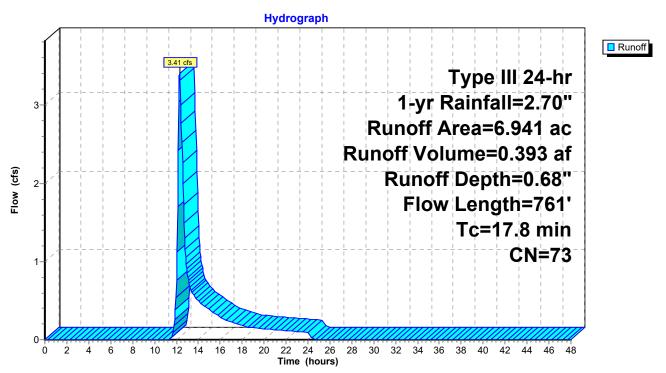
Routed to Pond 2P: East Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

	Area	(ac) C	N Desc	cription					
_	2.237 61			>75% Grass cover, Good, HSG B					
	3.	971 8	30 >759	% Grass c	over, Good	, HSG D			
	0.	210 9		el surface	•				
	0.	127 9		el surface	•				
_	0.	<u> 396 5</u>	55 Woo	ds, Good,	HSG B				
				ghted Aver					
	6.	941	100.	00% Pervi	ous Area				
	Τ.	1	01	V/-1	0	December them			
	Tc (min)	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	8.9	100	0.0244	0.19		Sheet Flow, grass			
						Grass: Short n= 0.150 P2= 3.32"			
	3.7	253	0.0259	1.13		Shallow Concentrated Flow, grass			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	22	0.0200	2.28		Shallow Concentrated Flow, road			
						Unpaved Kv= 16.1 fps			
	5.0	386	0.0340	1.29		Shallow Concentrated Flow, grass			
_						Short Grass Pasture Kv= 7.0 fps			
	17 8	761	Total						

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Subcatchment 207C: Subcat 207C



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 16

Summary for Subcatchment 207U: Subcat 207U

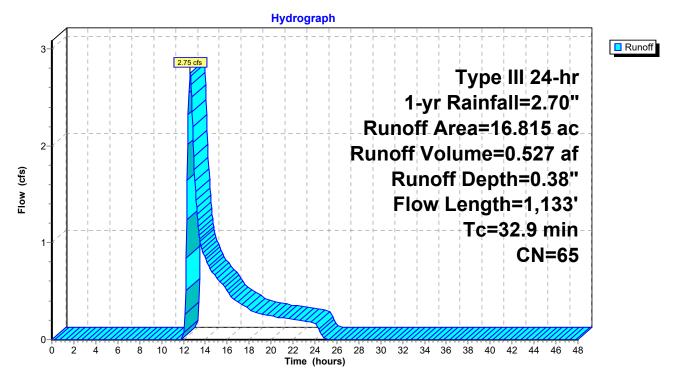
Runoff = 2.75 cfs @ 12.61 hrs, Volume= 0.527 af, Depth= 0.38" Routed to Link DP4 : Southern Wetland (Flags E, F, A, G, H)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 1-yr Rainfall=2.70"

Area	(ac) C	N Des	cription		
0.	838	61 >75°	% Grass co	over, Good,	HSG B
2.	238			over, Good,	
9.	014		ds, Good,		
4.			ds, Good,		
16.	815	65 Weid	ghted Aver	age	
_	815		00% Pervi		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
17.5	100	0.0318	0.10	, ,	Sheet Flow, woods
					Woods: Light underbrush n= 0.400 P2= 3.32"
6.8	303	0.0223	0.75		Shallow Concentrated Flow, woods
					Woodland Kv= 5.0 fps
2.7	280	0.0624	1.75		Shallow Concentrated Flow, grass
					Short Grass Pasture Kv= 7.0 fps
5.9	450	0.0654	1.28		Shallow Concentrated Flow, woods
					Woodland Kv= 5.0 fps
32.9	1,133	Total			

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Subcatchment 207U: Subcat 207U



Prepared by TRC Companies

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 18

Summary for Pond 1P: West Basin

Inflow Area = 1.711 ac. 1.03% Impervious, Inflow Depth = 1.09" for 1-yr event

Inflow 1.56 cfs @ 12.23 hrs, Volume= 0.155 af

Outflow 0.63 cfs @ 12.63 hrs, Volume= 0.155 af, Atten= 59%, Lag= 23.9 min

Discarded = 0.63 cfs @ 12.63 hrs, Volume= 0.155 af 0.00 cfs @ 0.00 hrs, Volume= Primary 0.000 af Routed to Link DP4: Southern Wetland (Flags E, F, A, G, H)

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 575.10' @ 12.63 hrs Surf.Area= 11,351 sf Storage= 1,180 cf

Plug-Flow detention time= 11.2 min calculated for 0.155 af (100% of inflow)

Center-of-Mass det. time= 11.2 min (868.9 - 857.7)

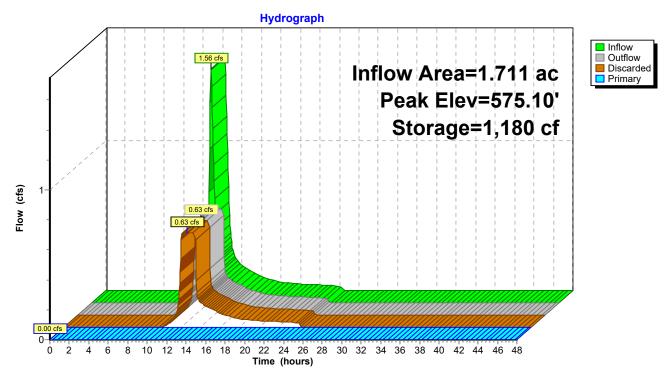
Volume	Invert	Avail.Sto	rage Storage	Description		
#1	575.00	11,9	54 cf Custom	Stage Data (Coni	i c) Listed below (Re	ecalc)
Elevation (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
575.0	00	11,195	0	0	11,195	
576.0	00	12,730	11,954	11,954	12,778	
Device	Routing	Invert	Outlet Devices	S		
#1	Discarded	575.00'		xfiltration over Su	rface area above	568.19'
				ace area = 0 sf		
#2	Primary	575.50'				Crested Rectangular Weir
			` ,	.20 0.40 0.60 0.8		1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50			
	Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65					2.65 2.65 2.65
		2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88				

Discarded OutFlow Max=0.63 cfs @ 12.63 hrs HW=575.10' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.63 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=575.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 19

Pond 1P: West Basin



Prepared by TRC Companies

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 20

Summary for Pond 2P: East Basin

Inflow Area = 6.941 ac, 0.00% Impervious, Inflow Depth = 0.68" for 1-yr event

Inflow = 3.41 cfs @ 12.28 hrs, Volume= 0.393 af

Outflow = 0.26 cfs @ 16.30 hrs, Volume= 0.393 af, Atten= 92%, Lag= 241.2 min

Discarded = 0.26 cfs @ 16.30 hrs, Volume= 0.393 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Link DP4 : Southern Wetland (Flags E, F, A, G, H)

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 552.43' @ 16.30 hrs Surf.Area= 21,441 sf Storage= 8,890 cf Flood Elev= 555.00' Surf.Area= 29,000 sf Storage= 73,563 cf

Plug-Flow detention time= 388.6 min calculated for 0.392 af (100% of inflow)

Center-of-Mass det. time= 388.7 min (1,277.1 - 888.4)

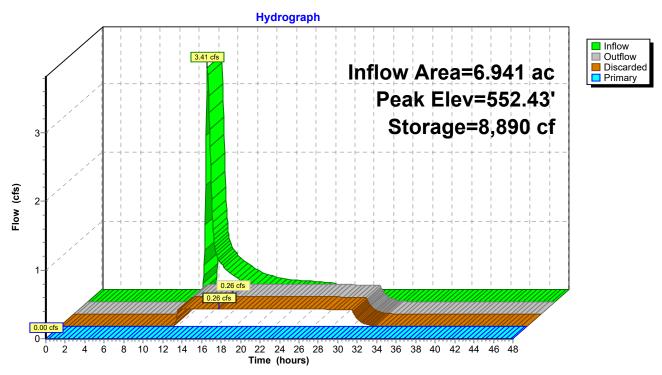
Volume	Inver	t Avail.Sto	rage Storage	Description		
#1	552.00	73,56	63 cf Custom	Stage Data (Coni	c) Listed below (Re	calc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
552.0 555.0		20,300 29,000	0 73,563	0 73,563	20,300 29,158	
Device	Routing	Invert	Outlet Device	S		
#1	Primary	554.00'	Head (feet) 0 2.50 3.00 3.5	0.20	0 1.00 1.20 1.40 5.50	
			, ,	n) 2.34 2.50 2.70 66 2.68 2.70 2.74		.65 2.65 2.65
#2	Discarded	552.00'				

Discarded OutFlow Max=0.26 cfs @ 16.30 hrs HW=552.43' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.26 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=552.00' (Free Discharge)
1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 21

Pond 2P: East Basin



Prepared by TRC Companies

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 22

Summary for Pond FB: Forebay

Inflow Area = 0.489 ac, 3.41% Impervious, Inflow Depth = 0.97" for 1-yr event

Inflow 0.40 cfs @ 12.22 hrs, Volume= 0.040 af

0.42 cfs @ 12.25 hrs, Volume= 0.42 cfs @ 12.25 hrs, Volume= Outflow 0.035 af, Atten= 0%, Lag= 1.7 min

Primary 0.035 af

Routed to Pond SF: Sand Filter

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 574.54' @ 12.25 hrs Surf.Area= 431 sf Storage= 203 cf

Flood Elev= 574.00' Surf.Area= 319 sf Storage= 0 cf

Plug-Flow detention time= 70.8 min calculated for 0.035 af (89% of inflow)

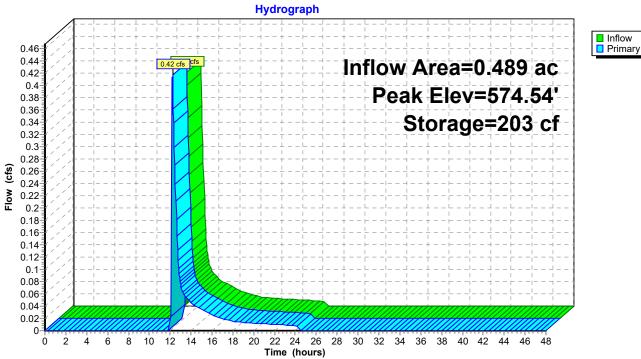
Center-of-Mass det. time= 19.8 min (883.8 - 864.1)

Volume	Inv	ert Avail.9	Storage St	orage De	escription		
#1	574.0	00'	424 cf C u	stom St	tage Data (Co	nic) Listed below	(Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Sto (cubic-fe	•	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
574.0	00	319		0	0	319	
575.0	00	539	4	24	424	551	
Device	Routing	Inve	ert Outlet D	evices			
#1	Primary	574.5	0' 14.0' lo	ng Shar	p-Crested Rec	tangular Weir 2	2 End Contraction(s)

Primary OutFlow Max=0.41 cfs @ 12.25 hrs HW=574.54' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.41 cfs @ 0.68 fps)

Page 23

Pond FB: Forebay





HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 24

Summary for Pond SF: Sand Filter

Inflow Area = 0.489 ac, 3.41% Impervious, Inflow Depth = 0.87" for 1-yr event

Inflow = 0.42 cfs @ 12.25 hrs, Volume= 0.035 af

Outflow = 0.05 cfs @ 13.66 hrs, Volume= 0.035 af, Atten= 88%, Lag= 84.6 min

Discarded = 0.05 cfs @ 13.66 hrs, Volume= 0.035 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Link DP2: Offsite B

Invort

Volume

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 573.34' @ 13.66 hrs Surf.Area= 2,018 sf Storage= 566 cf Flood Elev= 575.00' Surf.Area= 3,113 sf Storage= 3,565 cf

Plug-Flow detention time= 110.1 min calculated for 0.035 af (100% of inflow)

Avail Storage Storage Description

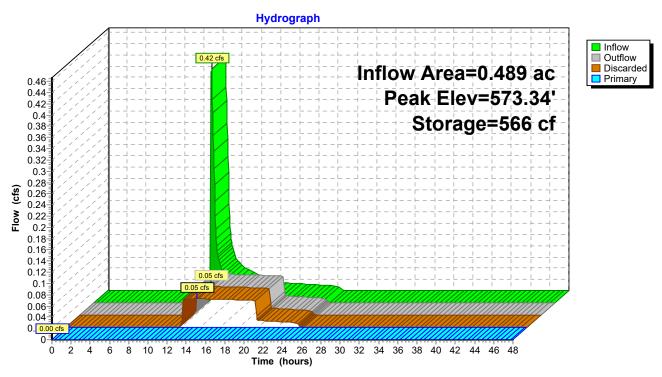
Center-of-Mass det. time= 110.0 min (993.9 - 883.8)

volume	mvert	Ava	ii.Storage	ge Storage Description			
#1	572.49'		7,238 cf	Custom Stage	Data (Conic)Listed	below (Recalc)	
Elevation	on S	urf.Area	Voids	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	(sq-ft)	
572.4	49	2,018	0.0	0	0	2,018	
572.5	50	2,018	33.0	7	7	2,020	
573.9	99	2,018	33.0	992	999	2,257	
574.0	00	2,018	100.0	20	1,019	2,258	
575.0	00	3,113	100.0	2,546	3,565	3,368	
576.0	00	4,264	100.0	3,673	7,238	4,539	
Device	Routing	In	vert Ou	tlet Devices			
#1	Discarded	572	2.49' 1.0	20 in/hr Exfiltration	on over Wetted ar	ea above 571.00'	
#2	Primary	575	5.50' 135	xcluded Wetted area = 0 sf 35.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir			
			Cv=	= 2.48 (C= 3.10)			

Discarded OutFlow Max=0.05 cfs @ 13.66 hrs HW=573.34' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=572.49' (Free Discharge) 2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Pond SF: Sand Filter



Page 26

Summary for Link DP1: Offsite A

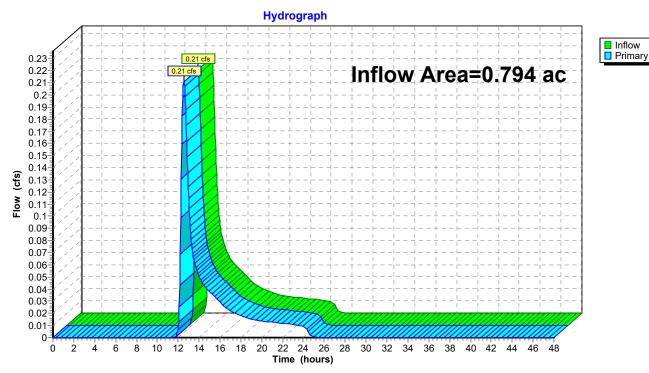
Inflow Area = 0.794 ac, 0.00% Impervious, Inflow Depth = 0.52" for 1-yr event

Inflow = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af

Primary = 0.21 cfs @ 12.55 hrs, Volume= 0.034 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP1: Offsite A



Page 27

Summary for Link DP2: Offsite B

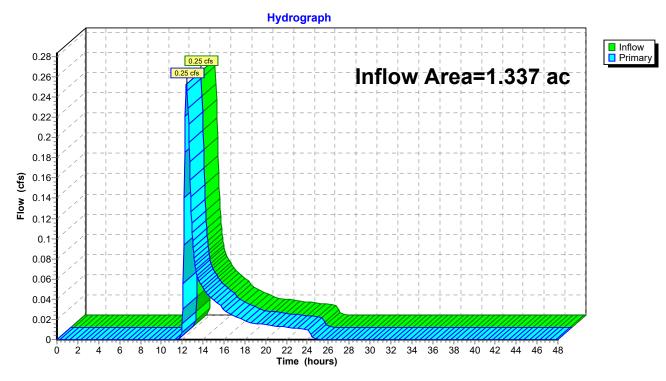
Inflow Area = 1.337 ac, 1.25% Impervious, Inflow Depth = 0.33" for 1-yr event

Inflow = 0.25 cfs @ 12.42 hrs, Volume= 0.036 af

Primary = 0.25 cfs @ 12.42 hrs, Volume= 0.036 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP2: Offsite B



Page 28

Summary for Link DP3: Wetland D

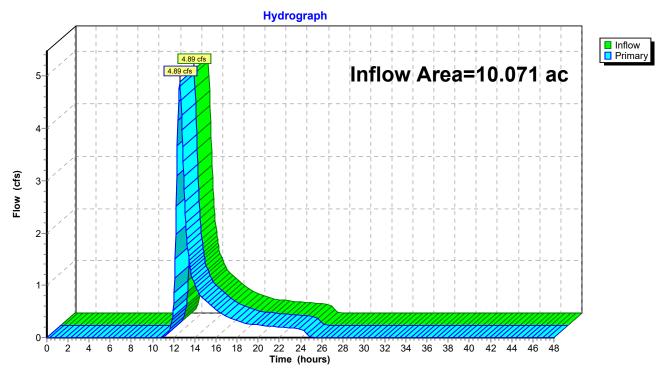
Inflow Area = 10.071 ac, 0.00% Impervious, Inflow Depth = 0.87" for 1-yr event

Inflow = 4.89 cfs @ 12.57 hrs, Volume= 0.729 af

Primary = 4.89 cfs @ 12.57 hrs, Volume= 0.729 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP3: Wetland D



HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Summary for Link DP4: Southern Wetland (Flags E, F, A, G, H)

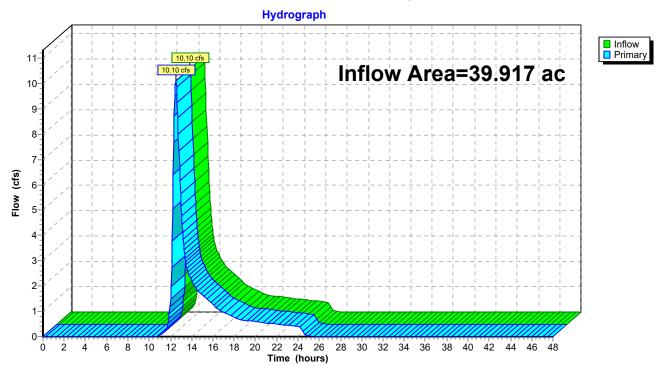
Inflow Area = 39.917 ac, 0.07% Impervious, Inflow Depth = 0.48" for 1-yr event

Inflow = 10.10 cfs @ 12.51 hrs, Volume= 1.596 af

Primary = 10.10 cfs @ 12.51 hrs, Volume= 1.596 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP4: Southern Wetland (Flags E, F, A, G, H)



Prepared by TRC Companies

Link DP1: Offsite A

Link DP2: Offsite B

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 1.2" Rainfall=1.20" Printed 4/15/2024

Inflow=0.00 cfs 0.001 af

Inflow=0.00 cfs 0.001 af Primary=0.00 cfs 0.001 af

Primary=0.00 cfs 0.001 af

Page 30

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

3 7 3	3 ,
Subcatchment201: Subcat 201	Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=0.02" Flow Length=379' Tc=32.2 min CN=69 Runoff=0.00 cfs 0.001 af
Subcatchment202: Subcat 202	Runoff Area=10.071 ac 0.00% Impervious Runoff Depth=0.10" Flow Length=870' Tc=38.0 min CN=77 Runoff=0.29 cfs 0.085 af
Subcatchment203: Subcat 203 Flow Lengt	Runoff Area=6.498 ac 0.14% Impervious Runoff Depth=0.10" h=448' Tc=36.6 min UI Adjusted CN=77 Runoff=0.19 cfs 0.055 af
Subcatchment204: Subcat 204	Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=0.10" Flow Length=295' Tc=21.3 min CN=77 Runoff=0.10 cfs 0.022 af
Subcatchment205C: Subcat 205C	Runoff Area=1.711 ac 1.03% Impervious Runoff Depth=0.17" Flow Length=224' Tc=16.0 min CN=81 Runoff=0.17 cfs 0.025 af
Subcatchment205U: Subcat 205U	Runoff Area=5.302 ac 0.02% Impervious Runoff Depth=0.12" Flow Length=283' Tc=29.4 min CN=78 Runoff=0.22 cfs 0.052 af
Subcatchment206C: Subcat 206C	Runoff Area=0.489 ac 3.41% Impervious Runoff Depth=0.13" Flow Length=185' Tc=15.3 min CN=79 Runoff=0.03 cfs 0.005 af
Subcatchment206U: Subcat 206U	Runoff Area=0.847 ac 0.00% Impervious Runoff Depth=0.02" Flow Length=128' Tc=24.2 min CN=69 Runoff=0.00 cfs 0.001 af
Subcatchment207C: Subcat 207C	Runoff Area=6.941 ac 0.00% Impervious Runoff Depth=0.05" Flow Length=761' Tc=17.8 min CN=73 Runoff=0.06 cfs 0.029 af
Subcatchment207U: Subcat 207U	Runoff Area=16.815 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=1,133' Tc=32.9 min CN=65 Runoff=0.01 cfs 0.004 af
Pond 1P: West Basin Discarded=0.16	Peak Elev=575.00' Storage=29 cf Inflow=0.17 cfs 0.025 af cfs 0.025 af Primary=0.00 cfs 0.000 af Outflow=0.16 cfs 0.025 af
Pond 2P: East Basin Discarded=0.05	Peak Elev=552.01' Storage=118 cf Inflow=0.06 cfs 0.029 af cfs 0.029 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.029 af
Pond FB: Forebay	Peak Elev=574.50' Storage=185 cf Inflow=0.03 cfs 0.005 af Outflow=0.00 cfs 0.001 af
Pond SF: Sand Filter Discarded=0.00	Peak Elev=572.49' Storage=0 cf Inflow=0.00 cfs 0.001 af cfs 0.001 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.001 af

Type III 24-hr 1.2" Rainfall=1.20"

Prepared by TRC Companies

Printed 4/15/2024

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 31

Link DP3: Wetland D Inflow=0.29 cfs 0.085 af

Primary=0.29 cfs 0.085 af

Link DP4: Southern Wetland (Flags E, F, A, G, H)

Inflow=0.49 cfs 0.133 af Primary=0.49 cfs 0.133 af

Total Runoff Area = 52.119 ac Runoff Volume = 0.280 af Average Runoff Depth = 0.06" 99.92% Pervious = 52.075 ac 0.08% Impervious = 0.044 ac

Prepared by TRC Companies

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 10-yr Rainfall=4.80" Printed 4/15/2024

Page 32

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment201: Subcat 201 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=1.81" Flow Length=379' Tc=32.2 min CN=69 Runoff=0.89 cfs 0.120 af

Subcatchment202: Subcat 202 Runoff Area=10.071 ac 0.00% Impervious Runoff Depth=2.46" Flow Length=870' Tc=38.0 min CN=77 Runoff=14.61 cfs 2.062 af

Subcatchment203: Subcat 203 Runoff Area=6.498 ac 0.14% Impervious Runoff Depth=2.46" Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=9.60 cfs 1.330 af

Subcatchment204: Subcat 204 Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=2.46" Flow Length=295' Tc=21.3 min CN=77 Runoff=4.99 cfs 0.543 af

Subcatchment205C: Subcat 205C Runoff Area=1.711 ac 1.03% Impervious Runoff Depth=2.81" Flow Length=224' Tc=16.0 min CN=81 Runoff=4.15 cfs 0.401 af

Subcatchment205U: Subcat 205URunoff Area=5.302 ac 0.02% Impervious Runoff Depth=2.54"
Flow Length=283' Tc=29.4 min CN=78 Runoff=9.00 cfs 1.123 af

Subcatchment206C: Subcat 206C Runoff Area=0.489 ac 3.41% Impervious Runoff Depth=2.63" Flow Length=185' Tc=15.3 min CN=79 Runoff=1.13 cfs 0.107 af

Subcatchment206U: Subcat 206U Runoff Area=0.847 ac 0.00% Impervious Runoff Depth=1.81" Flow Length=128' Tc=24.2 min CN=69 Runoff=1.08 cfs 0.128 af

Subcatchment207C: Subcat 207CRunoff Area=6.941 ac 0.00% Impervious Runoff Depth=2.12"
Flow Length=761' Tc=17.8 min CN=73 Runoff=12.03 cfs 1.229 af

Subcatchment207U: Subcat 207URunoff Area=16.815 ac 0.00% Impervious Runoff Depth=1.52"

Flow Length=1,133' Tc=32.9 min CN=65 Runoff=15.28 cfs 2.133 af

Pond 1P: West Basin

Peak Elev=575.51' Storage=5,909 cf Inflow=4.15 cfs 0.401 af

Discarded=0.67 cfs 0.397 af Primary=0.12 cfs 0.003 af Outflow=0.79 cfs 0.401 af

Pond 2P: East Basin

Peak Elev=553.74' Storage=39,569 cf Inflow=12.03 cfs 1.229 af

Discarded=0.30 cfs 0.889 af Primary=0.00 cfs 0.000 af Outflow=0.30 cfs 0.889 af

Pond FB: Forebay Peak Elev=574.58' Storage=221 cf Inflow=1.13 cfs 0.107 af

Outflow=1.13 cfs 0.103 af

Pond SF: Sand Filter

Peak Elev=574.64' Storage=2,510 cf Inflow=1.13 cfs 0.103 af

Discarded=0.07 cfs 0.103 af Primary=0.00 cfs 0.000 af Outflow=0.07 cfs 0.103 af

Link DP1: Offsite A Inflow=0.89 cfs 0.120 af Primary=0.89 cfs 0.120 af

Link DP2: Offsite B Inflow=1.08 cfs 0.128 af Primary=1.08 cfs 0.128 af

Type III 24-hr 10-yr Rainfall=4.80"

Prepared by TRC Companies

Printed 4/15/2024

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Page 33

Link DP3: Wetland D

Inflow=14.61 cfs 2.062 af Primary=14.61 cfs 2.062 af

Link DP4: Southern Wetland (Flags E, F, A, G, H)

Inflow=37.46 cfs 5.132 af Primary=37.46 cfs 5.132 af

Total Runoff Area = 52.119 ac Runoff Volume = 9.175 af Average Runoff Depth = 2.11" 99.92% Pervious = 52.075 ac 0.08% Impervious = 0.044 ac

Prepared by TRC Companies

HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Type III 24-hr 100-yr Rainfall=8.70" Printed 4/15/2024

Page 34

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Muskingum-Cunge method - Pond routing by Stor-Ind method

Subcatchment 201: Subcat 201 Runoff Area=0.794 ac 0.00% Impervious Runoff Depth=4.95" Flow Length=379' Tc=32.2 min CN=69 Runoff=2.52 cfs 0.328 af

Subcatchment202: Subcat 202 Runoff Area=10.071 ac 0.00% Impervious Runoff Depth=5.92"

Flow Length=870' Tc=38.0 min CN=77 Runoff=35.01 cfs 4.969 af

Runoff Area=6.498 ac 0.14% Impervious Runoff Depth=5.92" Subcatchment203: Subcat 203 Flow Length=448' Tc=36.6 min UI Adjusted CN=77 Runoff=23.02 cfs 3.206 af

Runoff Area=2.650 ac 0.00% Impervious Runoff Depth=5.92" Subcatchment 204: Subcat 204

Flow Length=295' Tc=21.3 min CN=77 Runoff=11.95 cfs 1.308 af

Subcatchment205C: Subcat 205C Runoff Area=1.711 ac 1.03% Impervious Runoff Depth=6.41" Flow Length=224' Tc=16.0 min CN=81 Runoff=9.27 cfs 0.913 af

Subcatchment 205U: Subcat 205U Runoff Area=5.302 ac 0.02% Impervious Runoff Depth=6.04" Flow Length=283' Tc=29.4 min CN=78 Runoff=21.18 cfs 2.669 af

Subcatchment206C: Subcat 206C Runoff Area=0.489 ac 3.41% Impervious Runoff Depth=6.16" Flow Length=185' Tc=15.3 min CN=79 Runoff=2.61 cfs 0.251 af

Subcatchment206U: Subcat 206U Runoff Area=0.847 ac 0.00% Impervious Runoff Depth=4.95"

Flow Length=128' Tc=24.2 min CN=69 Runoff=3.05 cfs 0.350 af

Subcatchment207C: Subcat 207C Runoff Area=6.941 ac 0.00% Impervious Runoff Depth=5.43" Flow Length=761' Tc=17.8 min CN=73 Runoff=31.09 cfs 3.143 af

Runoff Area=16.815 ac 0.00% Impervious Runoff Depth=4.47" Subcatchment207U: Subcat 207U Flow Length=1,133' Tc=32.9 min CN=65 Runoff=47.57 cfs 6.260 af

Peak Elev=575.66' Storage=7,716 cf Inflow=9.27 cfs 0.913 af Pond 1P: West Basin Discarded=0.68 cfs 0.617 af Primary=7.54 cfs 0.297 af Outflow=8.22 cfs 0.913 af

Pond 2P: East Basin Peak Elev=554.43' Storage=57,649 cf Inflow=31.09 cfs 3.143 af

Discarded=0.33 cfs 0.974 af Primary=15.24 cfs 1.678 af Outflow=15.57 cfs 2.652 af

Peak Elev=574.65' Storage=250 cf Inflow=2.61 cfs 0.251 af Pond FB: Forebay Outflow=2.61 cfs 0.247 af

Pond SF: Sand Filter Peak Elev=575.56' Storage=5,484 cf Inflow=2.61 cfs 0.247 af Discarded=0.09 cfs 0.204 af Primary=0.49 cfs 0.043 af Outflow=0.58 cfs 0.247 af

Link DP1: Offsite A Inflow=2.52 cfs 0.328 af Primary=2.52 cfs 0.328 af

Link DP2: Offsite B Inflow=3.05 cfs 0.393 af Primary=3.05 cfs 0.393 af

Type III 24-hr 100-yr Rainfall=8.70"

Prepared by TRC Companies
HydroCAD® 10.20-4b s/n 01402 © 2023 HydroCAD Software Solutions LLC

Printed 4/15/2024

Page 35

Link DP3: Wetland D Inflow=35.01 cfs 4.969 af

Primary=35.01 cfs 4.969 af

Link DP4: Southern Wetland (Flags E, F, A, G, H)

Inflow=118.31 cfs 15.417 af
Primary=118.31 cfs 15.417 af

Total Runoff Area = 52.119 ac Runoff Volume = 23.397 af Average Runoff Depth = 5.39" 99.92% Pervious = 52.075 ac 0.08% Impervious = 0.044 ac



Attachment B: Water Quality Calculations

Moo Cow Solar Coventry, RI DA 205C 1/22/2024

Water Quality Volume (WQV)				
Disturbed Area, DA, (ac)	1.66			
Impervious Area,IA, (ac)	0.018			
WQV, (cf)	64	IA * 1/12 * 43560		
Min WQV, (cf)	1,204	DA*0.2/12*43560		
Design WQV, (cf)	1,204	Max WQV		

West Basin					
Min Forebay Area Required (sf)	20	5,750 * 0.25WQV/86400			
Forebay Area Provided (sf)	330	165' L x 2' W			
Forebay Vol Rqd (cf)	301	0.25 * WQv			
Forebay Vol Provided (cf)	330	165' L x 2' W x 1' D			
Basin Vol Provided (cf)	5,785	From HydroCAD @ Elev 575.50'			

Total WQV provided (cf)	5,785	Vol. @ Elev. 575.50' includes Forebay

Moo Cow Solar Coventry, RI DA 205U 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	1.02	
Impervious Area,IA, (ac)	0.001	
WQV, (cf)	3	IA * 1/12 * 43560
Min WQV, (cf)	739	DA*0.2/12*43560
Design WQV, (cf)	739	Max WQV

Moo Cow Solar Coventry, RI DA 206C 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	0.47	
Impervious Area,IA, (ac)	0.017	
WQV, (cf)	61	IA * 1/12 * 43560
Min WQV, (cf)	345	DA*0.2/12*43560
Design WQV, (cf)	345	Max WQV

Sand Filter		
Min Forebay Area Required (sf)	6	5,750 * 0.25WQV/86400
Forebay Area Provided (sf)	319	
Forebay Vol Rqd (cf)	86.15	0.25 * WQv
Forebay Vol Provided (cf)	185	From HydroCAD @ Elev 574.50
Sand Filter Vol Required (cf)	258	0.75 * WQv
Sand Filter Vol Provided (cf)	5,258	From HydroCAD @ Elev 575.50

Total WQV provided (cf)	5,258	Vol. @ 575.50 includes Forebay

Moo Cow Solar Coventry, RI DA 206U 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	0.35	
Impervious Area,IA, (ac)	-	
WQV, (cf)	-	IA * 1/12 * 43560
Min WQV, (cf)	255	DA*0.2/12*43560
Design WQV, (cf)	255	Max WQV

Moo Cow Solar Coventry, RI DA 207C 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	6.54	
Impervious Area,IA, (ac)	-	
WQV, (cf)	-	IA * 1/12 * 43560
Min WQV, (cf)	4,751	DA*0.2/12*43560
Design WQV, (cf)	4,751	Max WQV

East Basin		
Min Forebay Area Required (sf)	79.05	5,750 * 0.25WQV/86400
Forebay Area Provided (sf)	1,269	423' L x 3' W
Forebay Vol Rqd (cf)	1187.83	0.25 * WQv
Forebay Vol Provided (cf)	1,269	423' L x 3' W x 1' D
Basin Vol Provided (cf)	46,113	From HydroCAD @ Elev 554

Total WQV provided (cf)	46,113	Vol. @ Elev. 554 includes forebay

Moo Cow Solar Coventry, RI DA 207U 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	3.08	
Impervious Area,IA, (ac)	-	
WQV, (cf)	-	IA * 1/12 * 43560
Min WQV, (cf)	2,233	DA*0.2/12*43560
Design WQV, (cf)	2,233	Max WQV

Moo Cow Solar Coventry, RI Design Point 2 1/22/2024

Water Quality Volume (WQV)		
Disturbed Area, DA, (ac)	0.83	
Impervious Area,IA, (ac)	0.017	
WQV, (cf)	61	IA * 1/12 * 43560
Min WQV, (cf)	599	DA*0.2/12*43560
Design WQV, (cf)	599	Max WQV

Sand Filter		
Min Forebay Area Required (sf)	9.97	5,750 * 0.25WQV/86400
Forebay Area Provided (sf)	319	
Forebay Vol Rqd (cf)	149.83	0.25 * WQv
Forebay Vol Provided (cf)	185	
Sand Filter Vol Provided (cf)	5,258	From HydroCAD @ Elev 575.50

Total WQV provided (cf)	5,258 Vol. @ 575.50 includes Forebay

Moo Cow Solar Coventry, RI Design Point 4 1/22/2024

Water Quality Volume (WQV)								
Disturbed Area, DA, (ac)	12.30							
Impervious Area,IA, (ac)	0.002							
WQV, (cf)	7	IA * 1/12 * 43560						
Min WQV, (cf)	8,927	DA*0.2/12*43560						
Design WQV, (cf)	8,927	Max WQV						

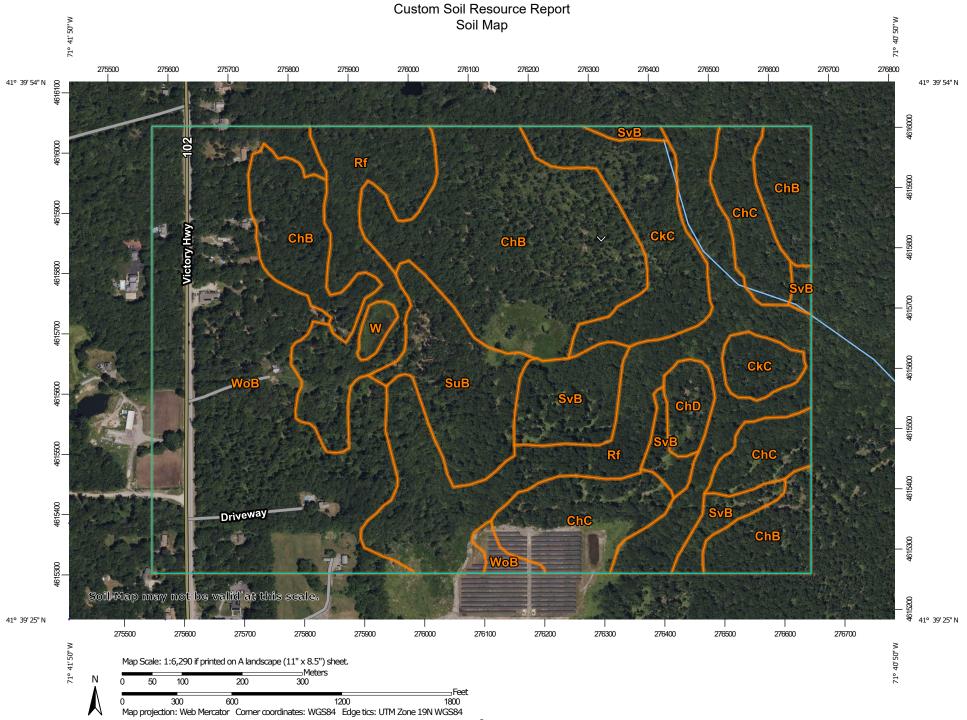
West Basin									
Min Forebay Area Required (sf)	148.53	5,750 * 0.25WQV/86400							
Forebay Area Provided (sf)	330								
Forebay Vol Rqd (cf)	2231.86	0.25 * WQv							
Forebay Vol Provided (cf)	330								
Basin Vol Provided (cf)	5,785	From HydroCAD @ Elev 554							

East Basin									
Min Forebay Area Required (sf)	148.53	5,750 * 0.25WQV/86400							
Forebay Area Provided (sf)	1,269								
Forebay Vol Rqd (cf)	2231.86	0.25 * WQv							
Forebay Vol Provided (cf)	1,269								
Basin Vol Provided (cf)	46,113	From HydroCAD @ Elev 575.50							

Total WQV provided (cf)	51,898	Vol. includes forebays



Attachment C: NRCS Soils Map





Attachment D: Test Pit Logs

1									
	T DIT 1	00	SITE:		Coventry RI	TEST PIT NO:	TP-1		
IE	ST PIT L	.OG	CLIEN	<u>T</u> :	Moo Cow Solar	DATE:	6/8/202	3	
			CONTI	RACTOR	Geosearch, Inc.	TRC INSPECTOR:	Michele	Grenie	er
*>	TR		METH	<u>OD</u> :	Mini-Excavator	SURFACE ELEV:	NM		
			WEAT	HER:	Partly Cloudy, 70° F	DEPTH TO WATER:	N/A		
Depth (feet)	Soil Texture	Hydr	RCS ologic Group/	Depth (feet)	Materials I	Description	Depth (feet)		
Dep	Class		s Rate	Del		, major and minor constituents	Dep	Unit	Notes
0.5	Loamy Sand		0.27	0.5	Moist, dark brown SILT AND O		0.5		
1.0	Loamy Sand	A/2	2.41	1.0	Dry, dark yellowish brown FINE and clay, trace gravel (contains		1.0		
1.5	Sandy	B/	1.02	1.5	Moist, yellowish brown FINE To		1.5		
2.0	Loam			2.0	some clay, trace gravel (contai	ns copples and boulders)	2.0		
2.5				2.5			2.5		
3.0	Sand	A/8	8.27	3.0	Wet, brown FINE TO COARSE		3.0		
3.5				3.5	(contains cobbles and boulders	5)	3.5	i .	
4.0				4.0			4.0	Native Soil	
4.5				4.5			4.5	Z	
5.0				5.0			5.0		
5.5				5.5			5.5		
6.0				6.0			6.0		
6.5 - 7.0 -				6.5 7.0			6.5 -		
7.5				7.5			7.5		
8.0				8.0			8.0		Groundwater Table
8.5				8.5	Excavation	Terminated	8.5		
9.0				9.0			9.0		
9.5				9.5			9.5		
10.0				10.0			10.0		
PROPO	DRTIONS L	JSED	MOIST	URE		COMMENTS:	•		•
Trace	<10%		Dry		NM = Not Measured	• A significant amount of large b	oulders a	and cob	bles encountered

10-20%

20-35%

35-50%

Damp

Moist

Wet

Little

Some

And

NA = Not Available

bgs = below ground

surface

from surface elevation to total depth.

No redoximorphic features observed .
Groundwater encountered at ~8 ft bgs.

Excavation terminated at ~8.0 ft bgs (limit of equipment).

TEST PIT LOG CLIEN CONTI METHO WEAT				RACTOR	Coventry RI Moo Cow Solar Geosearch, Inc. Mini-Excavator Partly Cloudy, 70° F	TEST PIT NO: DATE: TRC INSPECTOR: SURFACE ELEV: DEPTH TO WATER:	TP-2 6/8/202 Michele NM N/A	23 e Grenie	er
Depth (feet)	Soil Texture Class	Hydr Soil (RCS ologic Group/ s Rate	Depth (feet)		s Description ze, major and minor constituents	Depth (feet)	Unit	Notes
0.5 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 1.0	Silt Loamy Sand Sand	C/0	D.27 2.41 3.27	0.5 1.0	Moist, dark brown SILT AND Dry, dark yellowish brown Fll and clay, trace gravel (contain Dry, yellowish brown FINE Togravel (contains cobbles and	ORGANICS (roots) NE TO COARSE SAND, little silt ins cobbles and boulders) O COARSE SAND, trace silt and	1.0 1.5 2.0 2.5 3.0 3.5 4.0 5.0 5.5 6.0 7.5 8.0 9.0 9.5		
PROPO Trace Little Some And	CALCART STATE OF THE ACT OF TH	JSED	MOIST Dry Damp Moist Wet	URE	LEGEND: NM = Not Measured NA = Not Available bgs = below ground surface	• A significant amount of large befrom surface elevation to total de No redoximorphic features observos No groundwater encountered. • Excavation terminated at ~7.0	epth. served .		

TEST PIT LOG CLIENT CONTE				RACTOR	Coventry RI Moo Cow Solar Geosearch, Inc. Excavator	TEST PIT NO: DATE: TRC INSPECTOR: SURFACE ELEV:	TP-3 6/14/29 Craig F	023 Paradis	
	•••		WEATI		Partly Cloudy, 70° F	DEPTH TO WATER:	N/A		
Depth (feet)	Soil Texture Class	Hydro Soil G	RCS ologic Group/ s Rate	Depth (feet)		Description , major and minor constituents	Depth (feet)	Unit	Notes
0.5 1.0 1.5	Sandy Loam Loamy Sand	C/0	1.02	1.5	sand, little fine and coarse gra Moist to damp grayish brown N coarse sand, fine gravel, little o	ND AND SILT, trace medium e gravel, little fine gravel	0.5 _ 1.0	Native Soil	Seasonal High Groundwater
PROPO Trace Little Some And	CALCARTIONS L <10% 10-20% 20-35% 35-50%	<u>JSED</u>	MOIST Dry Damp Moist Wet	URE	NM = Not Measured NA = Not Available bgs = below ground surface	COMMENTS: • A significant amount of large before the surface elevation to total developments of the surface elevation to total developments of the surface	epth. ved at 2	.5 feet b	gs.

			ı								
TEG	ST PIT L	OG	SITE:			Coventry RI		TEST PIT NO:	TP-5		
ILC)	UG	CLIEN.	<u>r</u> :		Moo Cow Solar		DATE:	6/14/20	23	
			CONTR	RACT	OR:	Geosearch, Inc.		TRC INSPECTOR:	Craig P	aradis	
TRC METHO		<u>)D</u> :		Excavator		SURFACE ELEV:	NM				
			WEATI	HER:		Partly Cloudy, 70° F		DEPTH TO WATER:	N/A		
Depth (feet)	Soil Texture	NRCS Hydrologic Soil Group/		Depth (feet)		Materials Description		Depth (feet)			
Dep	Class		s Rate	Dep	ı	Moisture, Color, density, size			Dep	Unit	Notes
	Loamy	C/(0.27	Dry, dark brown SILT AND ORGANICS (roots)				_			
0.5	Sand			0.5	1	•		` ,	0.5		
1.0	Loam	B/0	0.52	1.0	an	oist to damp, brown to grayis nd medium sand, little coarse parse gravel (contains cobble	sand, l	ittle to trace fine and	1.0		
1.5				1.5					1.5		
2.0				2.0	=				2.0		
2.5				2.5	1				2.5		
3.0				3.0	4				3.0		Seasonal High Groundwater
3.5				3.5	1				3.5	=	
4.0				4.0_	1				4.0	Native Soil	
4.5				4.5	-				4.5	Na	
5.0				5.0	-				5.0		
5.5	Sandy Loam	B/	1.02	5.5		oist, brown FINE AND MEDI tle coarse and fine sand, cob		ND, some coarse gravel	5.5		
6.0				6.0	=				6.0		
6.5				6.5	1				6.5		
7.0				7.0	-				7.0		
7.5				7.5	1				7.5		
8.0				8.0_	1				8.0		
8.5				8.5	1	Re	fusal		8.5		
9.0				9.0	4				9.0		
9.5				9.5	1				9.5		
10.0				10.0	=				10.0		
lacksquare	ORTIONS L	JSED	MOIST	_		LEGEND:	СОММ	ENTS:			
Trace	<10%		Dry			NM = Not Measured		loximorphic features obs	erved.		
Little	10-20%		Damp			NA = Not Available		oundwater encountered.	h == /-		a adva alt)
Some And	20-35% 35-50%		Moist Wet			bgs = below ground surface	• Ketusa	al encountered at ~8.0 ft	ugs (ass	sumed b	реигоск).

TEST PIT LOG					Coventry RI	TEST PIT NO:	TP-6			
	, <u> </u>		CLIEN.	_	Moo Cow Solar	DATE:	6/14/20	23		
A	TR			RACTO	Geosearch, Inc.	TRC INSPECTOR:	Craig P	aradis		
	IR		METHO		Excavator	SURFACE ELEV:	NM			
			WEATI	HER:	Partly Cloudy, 70° F	DEPTH TO WATER:	~6 feet		_	
et)		NE	RCS	et)			et)			
Depth (feet)	Soil		ologic	Depth (feet)			Depth (feet)			
epth	Texture		Group/	ept		<u>Description</u>	epth	Unit		
۵	Class		s Rate		·	e, major and minor constituents	Ğ	ā	Notes	
	Silt Loam		0.27		Moist, dark brown SILT AND C]			
0.5	Loam	B/0	0.52	0.5	Moist, light brown FINE SAND little coarse sand, little to trace		0.5			
1.0				1.0	(contains cobbles and boulder		1.0			
···`=				" =			· · · · <u> </u>			
1.5				1.5			1.5			
7				=			-			
2.0				2.0			2.0			
<u>, </u>]			
2.5				2.5			2.5			
3.0	Sandy	B/1	1.02	3.0	Moist to wet, yellowish brown I	MEDIUM SAND, some fine	3.0		Seasonal High	
\exists	Loam	_,		=	sand, little silt, coarse sand, ar	nd fine and coarse gravel	=		Groundwater —	
3.5				3.5	(contains cobbles and boulder	s)	3.5			
=				=]	Native Soil		
4.0				4.0			4.0	ative		
4.5				4.5			4.5	ž		
4.3				4.5			4.5			
5.0				5.0			5.0			
5.5				5.5			5.5			
_]		Groundwater Table	
6.0				6.0			6.0			
6.5				6.5			6.5			
·· ∃				-			-			
7.0				7.0			7.0			
-				=			=			
7.5				7.5			7.5			
,]] =			
8.0				8.0	Ke	fusal	8.0			
8.5				8.5			8.5			
				_			-			
9.0				9.0			9.0			
= =] =			
9.5			ļ	9.5			9.5			
10.0				10.0			10.0			
	ORTIONS L	ISED	MOIST		LEGEND:	COMMENTS:				
Trace	<10%		Dry		NM = Not Measured	Redoximorphic features observed.	ved at 3.	2 feet b	ogs.	
Little	10-20%		Damp		NA = Not Available	• Groundwater in-flow at ~6.0 ft			h - dudu)	
Some And	20-35% 35-50%		Moist Wet		bgs = below ground surface	 Refusal encountered at ~7.8 ft 	pgs (ass	umed l	pearock).	
,	00 00 /0		7701		Saridoc					

TEST PIT LOG					Coventry RI	TEST PIT NO:	TP-7		
IES	I PII L	OG	CLIEN.	<u>r</u> :	Moo Cow Solar	DATE:	6/14/20	23	
			CONT	RACTOR:	Geosearch, Inc.	TRC INSPECTOR:	Craig Pa	aradis	
TRC METHO			METHO	<u>DD</u> :	Excavator	SURFACE ELEV:	NM		
			WEAT	HER:	Partly Cloudy, 70° F	DEPTH TO WATER:	~10.25	ft	
Depth (feet)	Soil Texture Class	Hydro Soil C	CS ologic Group/ s Rate	Depth (feet)		Description e, major and minor constituents	Depth (feet)	Unit	Notes
0.5	Sandy Loam	B/1.02			Moist, brown SILT AND FINE rganics (roots), trace coarse	SAND, some medium sand and sand	0.5		
1.0	Sandy Loam	B/1	1.02		Damp, light brown FINE SANE oarse sand and organics, tra	D, some silt, little medium and	1.0		
1.5					contains cobbles)	J	1.5		
2.0				2.0			2.0		
2.5				2.5			2.5		Seasonal High
3.0	Loamy Sand	A/2	2.41	_	, 0	ND COARSE SAND, some fine nd fine sand (contains cobbles)	3.0		Groundwater
3.5				3.5			3.5		
4.0	Sandy Loam	B/1	1.02	40	Moist, light brown FINE SAND and, fine and coarse gravel,), trace medium and coarse and silt (contains cobbles and	4.0		
4.5				4.5 = b	oulders)		4.5		
5.0				5.0			5.0	Native Soil	
5.5				5.5			5.5	Nat	
6.0	Loamy	A/2	2.41	6.0 - N	Moist to wet, gravish brown M	EDIUM SAND, some fine and	6.0		
6.5	Sand				oarse sand and fine and coal		6.5		
7.0				7.0			7.0		
7.5				7.5			7.5		
8.0				8.0			8.0		
8.5				8.5			8.5		
9.0				9.0			9.0		
9.5				9.5			9.5		
10.0				10.0			10.0		
PROPO	RTIONS L	<u>JSED</u>	MOIST	URE	LEGEND:	COMMENTS:		ii	
Trace	<10%		Dry		NM = Not Measured	Redoximorphic features observed.			-
Little	10-20%		Damp		NA = Not Available	 Significant in-flow of groundwa Test pit terminated at ~10.5 fee 		0.25 ft l	ogs.
Some And	20-35% 35-50%		Moist Wet		bgs = below ground surface	 Test pit terminated at ~10.5 fee 	et bys.		

And

TEST PIT LOG			SITE:		Coventry RI	TEST PIT NO:	TP-7										
IES	DI PII L	.UG	CLIEN.	<u>T</u> :	Moo Cow Solar	DATE:	6/14/20	23									
			CONT	RACTOR	R: Geosearch, Inc.	TRC INSPECTOR:	Craig P	aradis									
TRC METHOL WEATH					Excavator	SURFACE ELEV:											
	1117		WEATI	HER:	Partly Cloudy, 70° F	DEPTH TO WATER:	~10.25	ft									
Depth (feet)	Soil Texture Class	NRCS Hydrologic Soil Group/ Rawls Rate		Hydrologic Soil Group/		Hydrologic Soil Group/		Hydrologic Soil Group/		Hydrologic Soil Group/		Depth (feet)	Materials Dominication Moisture, Color, density, size, i		Depth (feet)	Unit	Notes
	Loamy Sand	A/2.41		1 3	Moist to wet, grayish brown MEDIUM SAND, some fine and coarse sand and fine and coarse gravel, little silt			Nativ e Soil	Groundwater Table								
10.5	Cana			10.5	Excavation		10.5	Zυ									
11.0				11.0			11.0										
11.5				11.5			11.5										
12.0_				12.0_			12.0										
12.5				12.5			12.5										
13.0				13.0			13.0										
13.5				13.5			13.5										
14.0				14.0			14.0										
14.5				14.5			14.5										
15.0_				15.0_			15.0										
15.5				15.5			15.5										
16.0				16.0			16.0										
16.5				16.5			16.5										
17. <u>0</u>				17.0_			17.0										
17.5				17.5			17.5										
18.0				18.0			18.0										
18.5				18.5			18.5										
19.0				19.0			19.0										
19.5				19.5			19.5										
20. <u>0</u>				20. <u>0</u>			20.0										
PROPO	ORTIONS L	<u>JSED</u>	MOIST	URE		OMMENTS:											
Trace	<10%		Dry			Redoximorphic features obser											
Little Some	10-20% 20-35%		Damp Moist			Significant in-flow of groundwa Test pit terminated at ~10.5 fe		U.25 ft	bgs.								
And	20-35% 35-50%		Wet		surface	root pit tominated at ~ 10.5 le	or bys.										



Attachment E: Drainage Area Maps

